

SOLITUDE RETREAT HOME - LOT 1









CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH ALL **BUILDING CODE REQUIREMENTS**







One complete legible set of the plans, specs, and supplemental information with the county stamp, **PRINTED TO FULL SIZE** is required to be on site for all inspections.

GRAPHIC SYMBOLS/MATERIALS LEGEND

- \$ -	FLOOR OR POINT ELEVATION
< \$ \$	KEY NOTE SPECIFICATION KEY NOTE
B1	WALL TYPE
XXX	DOOR NUMBER
XX	WINDOW NUMBER
X-XX	FIXTURE TAG
X	REVISION TAG
I A101	DETAIL
	INTERIOR ELEVATION
	BUILDING ELEVATION
Room name	ROOM NAME & NUMBER
I A101	BUILDING SECTION
1 A101	WALL SECTION
£	CENTER LINE

	MASTER GRID LINES
A	Parking grid lines
	BUILDING GRID LINES
	E.I.F.S.
	CONCRETE MASONRY UNIT
	BRICK VENEER
	CUNCREIE GYPSIIM BOARD OR GROUT
	MORTAR
	BATT INSULATION
	RIGID INSULATION
	PLYWOOD
	ROUGH WOOD-CONTINUOUS
	ROUGH WOOD-BLOCKING
	WOOD TRIM
	STEEL
	GRAVEL

EARTH

	DKAWIN INDEX GENERAL		DEX INDEX STRUCTURAL
<u> </u>		\$101	
C001		\$102	CENERAL STRUCTURAL NOTES
G001	BUILDING AREA ANALYSIS	\$102	GENERAL DETAILS
0002		\$104	GENERAL DETAILS (CONT.)
		\$105	GENERAL DETAILS (CONT.)
	INDEX ARCHITECTURAL	S201	BASEMENT FOOTING & FOUNDATION PLAN
A 101		\$202	MAIN LEVEL FOOTING & FOUNDATION PLAN
A101		S301	FOOTING & FOUNDATION DETAILS
A102		S401	BASEMENT FLOOR FRAMING
A103		S402	ROOF FRAMING PLAN
A104		\$501	FRAMING DETAILS
A107		\$502	FRAMING DETAILS
A100		\$503	FRAMING DETAILS
A201		S601	PERSPECTIVE VIEWS
A201		S701	SIMPSON STRONG-WALL DETAILS
A203		\$702	SIMPSON STRONG- WALL DETAILS (CONT.)
A301	BUILDING SECTIONS	S703	SIMPSON STRONG- WALL DETAILS (CONT.)
A302	BUILDING SECTIONS	S704	SIMPSON STRONG- WALL DETAILS (CONT.)
A303	BUILDING SECTIONS	_	
A304	BUILDING SECTIONS	_	INDEX MECHANICAL
A305	BUILDING SECTIONS	M101	LEVEL 0 - MECHANICAL PLAN
A401	FIREPLACE ELEVATIONS & SECTIONS	M102	LEVEL 1 - MECHANICAL PLAN
A402	ENLARGED STAIRS		1
A403	STAIR & RAILING DETAILS		
A501	BUILDING DETAILS		INDEX ELECTRICAL
A502	BUILDING DETAILS	E101	LEVEL 0 - ELECTRICAL PLAN
A601	DOOR SCHEDULE, ELEVATIONS & DETAILS	E102	LEVEL 1 - ELECTRICAL PLAN
A602	WINDOW DETAILS		
A603	WINDOW ELEVATIONS		
A604	WINDOW ELEVATIONS		
		1	



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SHEET NUMBER:



PROJECT GENERAL NOTES AND CODE REQUIREMENTS - RESIDENTIAL R-3 (FOR SINGLE DWELLINGS

- 1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE 2015 INTERNATIONAL RESIDENTIAL CODE (I.R.C.). THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL SUB
- CONTRACTORS TO MEET THESE REQUIREMENTS. 2. THE CONTRACTOR SHALL BE RESPONSIBLE TO FIELD VERIFY ALL EXISTING CONDITIONS,
- UTILITIES, MEASUREMENTS, CONNECTIONS, ETC. 3. THE CONTRACTOR SHALL COMPLY WITH ALL NATIONAL, STATE, LOCAL, AND RELATED
- CODES AND STANDARD CONSTRUCTION PRACTICES.
- 4. CONTRACTOR SHALL BE RESPONSIBLE TO COORDINATE WITH GENERAL ENERGY NOTES AND/OR MODEL ENERGY CODE.
- 5. CONTRACTOR SHALL REPORT ANY DISCREPANCIES IN THE PLANS TO THE ARCHITECT PRIOR TO COMMENCING RELATED WORK.
- 6. AN APPROVED NUMBER OR ADDRESS SHALL BE PROVIDED FOR ALL NEW BUILDINGS IN SUCH A POSITION AS TO BE PLAINLY VISIBLE AND LEGIBLE FROM THE STREET OR ROAD FRONTING THE PROPERTY. SEE I.R.C. SECTION R319.
- 7. PROTECT WOOD AGAINST DECAY AS NOTED AND REQUIRED BY CODE. WHERE PROTECTION IS REQUIRED, WOOD MUST BE APPROVED TREATED OR DECAY RESISTANT (I.R.C. R317.1). SEE I.R.C. SECTION R317 & LOCAL JURISDICTION'S REGULATIONS AS REQUIRED BY IRC. ADDITIONAL REQUIREMENTS AS FOLLOWS:
 - A) WHEN WOOD JOISTS OR THE BOTTOM OF WOOD STRUCTURAL FLOORS ARE LOCATED CLOSER THAN 18 INCHES OR WOOD GIRDERS ARE LOCATED CLOSER THAN 12 INCHES TO EXPOSED GROUND IN CRAWL SPACES OR UNEXCAVATED AREAS LOCATED WITHIN THE PERIPHERY OF THE BUILDING FOUNDATION, PROTECTION IS REQUIRED. (I.R.C. R317.1 (1)) THE FLOOR ASSEMBLY, INCLUDING POSTS, GIRDERS, JOISTS AND SUBFLOOR, SHALL BE APPROVED WOOD OF NATURAL RESISTANCE TO DECAY (AS LISTED IN I.R.C.) OR TREATED WOOD. UNDER FLOOR AREAS SHALL BE PROVIDED WITH AN ACCESS AS PER I.R.C. SECTION R408.2.
 - B) FOUNDATION PLATES OR SILLS AND SLEEPERS ON A CONCRETE OR MASONRY SLAB, WHICH IS IN DIRECT CONTACT WITH EARTH, AND SILLS WHICH REST ON CONCRETE OR MASONRY FOUNDATIONS, SHALL BE TREATED WOOD OR FOUNDATION REDWOOD, ALL MARKED OR BRANDED BY AN APPROVED AGENCY. (I.R.C. R317.2). WHERE NOT SUBJECT TO WATER SPLASH OR TO EXTERIOR MOISTURE AND LOCATED ON CONCRETE HAVING A MINIMUM THICKNESS OF 3 INCHES WITH AN IMPERVIOUS MEMBRANE INSTALLED BETWEEN CONCRETE AND EARTH, THE WOOD MAY BE UNTREATED AND OF ANY SPECIES. C) COLUMNS AND POSTS LOCATED ON CONCRETE OR MASONRY FLOORS OR DECKS
 - EXPOSED TO THE WEATHER OR TO WATER SPLASH OR IN BASEMENTS AND WHICH SUPPORT PERMANENT STRUCTURES SHALL BE SUPPORTED BY CONCRETE PIERS OR METAL PEDESTALS PROJECTING ABOVE FLOORS UNLESS APPROVED WOOD OF NATURAL RESISTANCE TO DECAY OR TREATED WOOD IS USED. (I.R.C. R317.4 (1)). THE PEDESTALS SHALL PROJECT AT LEAST 6 INCHES ABOVE EXPOSED EARTH AND AT LEAST 1 INCH ABOVE SUCH FLOORS. INDIVIDUAL CONCRETE OR MASONRY PIERS SHALL PROJECT AT LEAST 8 INCHES ABOVE EXPOSED GROUND UNLESS THE COLUMNS OR POSTS WHICH THEY SUPPORT ARE OF APPROVED WOOD OF NATURAL RESISTANCE TO DECAY OR TREATED WOOD IS USED.
 - D) WOOD USED IN CONSTRUCTION OF PERMANENT STRUCTURES AND LOCATED NEARER THAN 8 INCHES TO EARTH SHALL BE TREATED WOOD OR WOOD OF NATURAL RESISTANCE TO DECAY, AS DEFINED IN I.R.C. WHERE LOCATED ON CONCRETE SLABS PLACED ON EARTH, WOOD SHALL BE TREATED WOOD OR WOOD OF NATURAL RESISTANCE TO DECAY. (I.R.C. R317.1 (3)).
 - E) WOOD FURRING OR FRAMING ATTACHED DIRECTLY TO THE INTERIOR OF EXTERIOR MASONRY OR CONCRETE WALLS BELOW GRADE EXCEPT WHERE AN APPROVED BARRIER IS INSTALLED BETWEEN THE WALL AND THE WOOD, SHALL BE TREATED OR RESISTANT TO DECAY. (I.R.C. R317.1 (7)).
- 8. JOISTS UNDER AND PARALLEL TO BEARING PARTITIONS SHALL BE SIZED PER ENGINEER, OR AT MINIMUM DOUBLE JOISTS. WHEN USING FLOOR TRUSSES USES 2 X 4 BLOCKING AT 24" O/C. MEET REQUIREMENTS PER IRC 502.4.
- 9. BRACE ALL EXTERIOR WALLS AND CROSS STUD PARTITIONS AT EACH END OF THE BUILDING AND AT LEAST EVERY 25'-0" OF LENGTH BY ONE OF THE FOLLOWING. A) APPROVED STRUCTURAL SHEATHING OF A MINIMUM THICKNESS OF 7/16". COORDINATE WITH SHEAR WALL SCHEDULE B) FOR ADDITIONAL BRACED WALL PANEL CONSTRUCTION OPTIONS, EXCEPTIONS AND RESTRICTIONS SEE I.R.C SECTION R602.10. COORDINATE W/ STRUCTURAL FOR SEISMIC
- AND ANY SPECIAL REQUIREMENTS. 10. ALL STEEL COLUMNS IN WALLS SHALL RECEIVE 1/2" DIAMETER THREADED BOLTS WELDED TO THE COLUMN AT 2'-0" O/C VERTICAL. STUD WALLS SHALL START AND STOP AT COLUMN AND BOLT TO COLUMN. BOLTS SHALL EXTEND THROUGH TWO STUDS MINIMUM AT ALL LOCATIONS EXCEPT AT WINDOWS AT EXTERIOR WALL, BOLTS MAY EXTEND THROUGH ONE
- 11. FIRE BLOCKING SHALL BE CONSTRUCTED OF 2" NOMINAL LUMBER OR (2) THICKNESS OF 1" NOMINAL LUMBER WITH BROKEN LAP JOINTS OR OTHER MATERIALS APPROVED OR TESTED. INSTALLED PER I.R.C., FIRE BLOCKING SHALL BE PROVIDED AT LOCATIONS AS FOLLOWS. SEE I.R.C. SECTION R302.11 AND R302.11.1

STUD.

- A) IN CONCEALED SPACES OF STUD WALLS AND PARTITIONS, INCLUDING FURRED SPACES, AT THE CEILING AND FLOOR LEVELS AND AT 10-FOOT INTERVALS BOTH VERTICAL AND HORIZONTAL. (I.R.C. R302.11 (1))
- B) AT ALL INTERCONNECTIONS BETWEEN CONCEALED VERTICAL AND HORIZONTAL SPACES SUCH AS OCCUR AT SOFFITS, DROP CEILINGS AND COVE CEILINGS. (I.R.C. R302.11 (2)) C) IN CONCEALED SPACES BETWEEN STAIR STRINGERS AT THE TOP AND BOTTOM OF THE
- RUN AND BETWEEN STUDS ALONG AND IN LINE WITH THE RUN OF STAIRS IF THE WALLS UNDER THE STAIRS ARE UNFINISHED. (I.R.C. R302.7). SEE I.R.C. R302.7 FOR ENCLOSURES UNDER STAIRS.
- D) IN OPENINGS AROUND VENTS, PIPES, DUCTS, CHIMNEYS, FIREPLACES AND SIMILAR OPENINGS WHICH AFFORD A PASSAGE FOR FIRE AT CEILING AND FLOOR LEVELS, WITH NON COMBUSTIBLE MATERIALS. (I.R.C. R302.11 (4)) E) AT OPENINGS BETWEEN ATTIC SPACES AND CHIMNEY CHASES FOR FACTORY-
- BUILT CHIMNEYS. (I.R.C. R1003.19) F) WHERE WOOD SLEEPERS ARE USED FOR LAYING WOOD FLOORING ON MASONRY OR
- CONCRETE FIRE-RESISTIVE FLOORS, THE SPACE BETWEEN THE FLOOR SLAB AND THE UNDERSIDE OF THE WOOD FLOORING SHALL BE FILLED WITH NON COMBUSTIBLE MATERIAL OR FIRE BLOCKED IN SUCH A MANNER THAT THERE WILL BE NO OPEN SPACES UNDER THE FLOORING WHICH WILL EXCEED 100 SQUARE FEET IN AREA AND SUCH SPACE SHALL BE FILLED SOLIDLY UNDER ALL PERMANENT PARTITIONS SO THAT THERE IS NO COMMUNICATION UNDER THE FLOORING BETWEEN ADJOINING ROOMS.
- G) WALLS HAVING PARALLEL OR STAGGERED STUDS FOR SOUND TRANSMISSION CONTROL SHALL HAVE FIRE BLOCKS OF MINERAL OR GLASS FIBER OR OTHER APPROVED NON-RIGID MATERIAL. (I.R.C. R302.11.1.1)
- 12. CRAWL SPACE ACCESS AND ATTIC ACCESS TO MEET THE FOLLOWING REQUIREMENTS. SEE I.R.C. SECTION R408 AND SECTION R807. A) CRAWL SPACE ACCESS SHALL BE MINIMUM 18 INCH BY 24 INCH OPENING UNOBSTRUCTED BY PIPES, DUCTS AND SIMILAR CONSTRUCTION. ALL UNDER-FLOOR
- ACCESS OPENINGS SHALL BE EFFECTIVELY SCREENED OR COVERED. PIPES, DUCTS AND OTHER CONSTRUCTION SHALL NOT INTERFERE WITH THE ACCESSIBILITY TO OR WITHIN UNDER-FLOOR AREAS. WHERE MECHANICAL EQUIPMENT IS LOCATED UNDER FLOORS SEE I.R.C. SECTION M1305.1.4 FOR ACCESS REQUIREMENTS. SEE I.R.C. SECTION R408.4. B) ATTIC ACCESS OPENING SHALL BE PROVIDED TO ATTICS OF BUILDINGS WITH
- COMBUSTIBLE CEILING OR ROOF CONSTRUCTION THAT EXCEED 30 SQUARE FEET AND HAVE A VERTICAL HEIGHT OF 30 INCHES OR GREATER. THE OPENING SHALL BE LOCATED IN A CORRIDOR, HALLWAY OR OTHER READILY ACCESSIBLE LOCATION. THE ROUGH FRAME OPENING SHALL NOT BE LESS THAN 22 INCHES X 30 INCHES. A 30 INCH MINIMUM UNOBSTRUCTED HEADROOM IN THE ATTIC SPACE SHALL BE PROVIDED ABOVE THE OPENING. SEE I.R.C. SECTION R807. FOR ACCESS REQUIREMENTS WHERE MECHANICAL EQUIPMENT IS LOCATED IN ATTICS SEE I.R.C. SECTION M1305.1.3
- 13. ROOF AND UNDER FLOOR VENTILATION SHALL MEET THE FOLLOWING REQUIREMENTS. SEE I.R.C. SECTION R408 AND SECTION R806. A) UNDER FLOOR AREAS SHALL BE VENTILATED BY OPENINGS INTO THE UNDER FLOOR AREA WALLS. SUCH OPENINGS SHALL HAVE A NET AREA OF NOT LESS THAN 1 SQUARE FOOT FOR EACH 150 SQUARE FEET OF UNDER FLOOR AREA. ONE SUCH VENTILATION OPENING SHALL BE WITHIN 3 FEET OF EACH CORNER OF THE BUILDING. THE OPENINGS SHALL BE COVERED WITH
 - CORROSION RESISTANT WIRE MESH WITH MESH OPENINGS OF 1/8 INCH IN DIMENSION OR OTHER APPROVED MATERIALS AS PER I.R.C. TWO REFERENCE OF EXCEPTIONS ARE AS FOLLOWS. 1) THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 1/1,500 OF UNDER FLOOR AREA WHERE GROUND SURFACE IS TREATED WITH AN APPROVED
 - VAPOR BARRIER AND THE REQUIRED OPENINGS ARE PLACED SO AS TO CREATE A CROSS VENTILATION OF THE SPACE. SEE I.R.C. SECTION 408.2. 2) UNDER FLOOR AREAS VENTILATED BY AN APPROVED MECHANICAL MEANS AT A RATE OF 1.0 CFM FOR EACH 50 SQUARE FEET OF UNDER FLOOR SPACE, CONTINUOUSLY OPERATED, AND THE GROUND SURFACE IS COVERED WITH AN APPROVED BARRIER. SEE I.R.C. R408.3 (2).
 - B) ROOF VENTILATION: ENCLOSED ATTICS AND ENCLOSED RAFTER SPACES FORMED WHERE CEILINGS ARE APPLIED DIRECTLY TO THE UNDERSIDE OF ROOF RAFTERS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN AND SNOW. VENTILATION OPENINGS SHALL HAVE A LEAST DIMENSION OF 1/16 INCH MINIMUM AND 1/4 INCH MAXIMUM. VENTILATION OPENINGS HAVING A LEAST DIMENSION LARGER THAN 1/4 INCH SHALL BE PROVIDED WITH CORROSION RESISTANT WIRE CLOTH SCREENING, HARDWARE CLOTH, OR
- SIMILAR MATERIAL WITH OPENINGS HAVING A LEAST DIMENSION OF 1/16 INCH MINIMUM AND 1/4 INCH MAXIMUM. OPENINGS IN ROOF FRAMING MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF SECTION R802.7 C) MINIMUM AREA: THE TOTAL NET FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/150 OF THE AREA OF THE SPACE VENTILATED EXCEPT THAT REDUCTION OF THE TOTAL
- AREA TO 1/300 IS PERMITTED PROVIDED THAT AT LEAST 50 PERCENT AND NOT MORE THAN 80 PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED AT LEAST 3 FEET (914 mm) ABOVE THE EAVE OR CORNICE VENTS WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS. AS AN ALTERNATIVE, THE NET FREE CROSS-VENTILATION AREA MAY BE REDUCED TO 1/300 WHEN A CLASS I OR II VAPOR BARRIER IS INSTALLED ON THE WARM-IN-WINTER SIDE OF THE CEILING.

- 14. EMERGENCY ESCAPE AND RESCUE OPENINGS SHALL MEET THE FOLLOWING REQUIREMENTS. SEE I.R.C. SECTION R310. A) BASEMENTS WITH HABITABLE SPACES SHALL HAVE AT LEAST ONE OPERABLE

 - MINIMUM NET CLEAR OPENING WIDTH SHALL BE 20 INCHES. EMERGENCY OPENINGS
 - OPENINGS WITH A FINISHED SILL HEIGHT BELOW THE ADJACENT GROUND ELEVATION SHALL BE PROVIDED WITH A WINDOW WELL. R310.1

 - F) BARS, GRILLS, COVERS, SCREENS, ETC. SHALL BE PERMITTED TO BE PLACED OVER THE REQUIRED FOR NORMAL OPERATION. R310.4



17. HANDRAILS SHALL MEET THE FOLLOWING REQUIREMENTS. SEE I.R.C. SECTION R311.7.8.

- 36 INCHES MEASURED IN THE DIRECTION OR TRAVEL. FOR LANDINGS WITH ADJOINING DOORS SEE I.R.C. SECTION R311.7.6 FOR SOME EXCEPTIONS/OPTIONS. D) ENCLOSED ACCESSIBLE SPACE UNDER STAIRS SHALL HAVE WALLS, UNDER STAIR SURFACE AND ANY SOFFITS PROTECTED ON THE ENCLOSED SIDE WITH MINIMUM 1/2 INCH
- GYPSUM BOARD. (I.R.C. R302.7) E) HEADROOM: EVERY STAIRWAY SHALL HAVE A MINIMUM HEADROOM CLEARANCE IN ALL PARTS OF THE STAIR OF NOT LESS THAN 6 FEET 8 INCHES. SUCH CLEARANCES SHALL BE MEASURED VERTICALLY FROM THE SLOPED PLANE ADJOINING THE TREAD NOSING OR FROM THE FLOOR SURFACE OF THE LANDING. (I.R.C. R311.7.2)

ASSEMBLIES, AIRBORNE STC 45, & IMPACT IIC 45. 31. WHERE PROVIDED, SPARK ARRESTORS MEET IRC R1003.9.2, WITH NET FREE AREA 4X CHIMNEY FLUE OUTLET, ARRESTOR SCREEN TO BE HEAT AND CORROSION RESISTANT, THE SCREEN SHALL PERMIT SPHERES LESS THAN 3/8" BUT NOT GREATER THAN 1/2", AND THE ARRESTOR SHALL BE ACCESSIBLE FOR CLEANING AND THE SCREEN OR CHIMNEY CAP SHALL BE REMOVABLE. 32. WHERE ACCESSIBILITY GRAB BARS ARE PROVIDED, INSTALL SOLID BLOCKING IN WALLS

FINISH GRADE

CROSS SECTION

12-INCHES AT BASE.

FOR ATTACHMENT. COORDINATE WITH PLANS FOR LOCATIONS.

WIDTH OF STRAW BALE







WATER HEATER SIESMIC STRAPPING

1/2" = 1'-0"



Architecture Interior Design Landscape Architecture Land Planning Construction Managemen

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Plot Date: 8/26/2019 11:05:11 AM



	AREA - FINISHED	
NAME	AREA	
LEVEL 0 - FINISHED		
LEVEL 1 - FINISHED		
Grand total: 2		
Α	REA - UNFINISHED	
NAME	AREA	
GARAGE		
LEVEL 0 - MECH/STOR		-
Grand total: 2		



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8. COORDINATE WITH ELECTRCIAL	DRAWINGS FOR ALL L	IGHTING, POWER A	ND DATA REQUIREMENTS.

CEILING	PI A			
HATCH PATTERN		DESCRIPTION	HATCH PATTERN	DESCRIPTION
		CT-1 1 Layer - 5/8" Water Proofed Gypsum Board, finish as Selected		
		CT-2 1 LAYER 5/8" GYPSUM BOARD, FINISH AS SELECTED.		
		CT-3 1 LAYER SUSPENDED 5/8" GYPSUM BOARD ATTACHED TO STUDS @ 16" O.C., FINISH AS SELECTED		
		CT-4 1 LAYER SUSPENDED 5/8" GYPSUM BOARD FIRE RATED (TYPE 'X')		
		CT-5 TILE FINISH OVER TILE BACKER BOARD, COORD. W/ INT. DESIGNER		
		CT-6 VENTED ROOF SOFFIT, SEE DETAILS		
			<u>C1</u> 1' - 0"	
NOTE			C1 SLOPE	
ALL INTERIOR F SPECIFICATION	INISHES IS, COLC	ARE NOTED FOR CONCEPT ONLY. DRS, PATTERNS, AND OTHER REQUI	SEE INTERIOR DESIGN DRAV REMENTS PRIOR TO INSTALLA	VINGS FOR MATERIAL NION.
		PROJECT	KEYNOTES	
05-02	STRUC SELEC	CTURAL STEEL BEAMS, SEE ST		S AND DETAILS & FINISH AS
09-03 26-02A	S/8" F	TURES SHALL HAVE A U.L.	M BOARD, FINISH AS S LABEL LISTING.	SELECTED. SEE DETAILS.
26-02B		AMPS PERMANENILY INSTA ER.		MPS AT K TEMP. AS SEL. BY
26-04A CG-05	ALL C EXPO	usiom fixiures shall ha sed structural beam, sei	e structural drawi	G. NGS.

REFLECTED CEILING PLAN GENERAL NOTES

1. ALL DIMENSIONS ARE TO INTERIOR FACE-OF-STUD (F.O.S.) UNLESS NOTED OTHERWISE.

2. CEILING HEIGHTS MEASURED FROM TOP OF PLYWOOD OR CONCRETE - SEE SECTIONS

3. REFER TO ENLARGED PLANS FOR ALL UNIT DIMENSIONS, WINDOW TYPES, DOORS AND WALLS.

4. REFER TO ENLARGED PLANS FOR ALL DECKS.

A109

5. COORDINATE WITH ALL ENLARGED PLANS FOR ADDITIONAL INFORMATION AND DETAILS.

6. ALL TOPPING SLABS MUST BE POURED AFTER ROOF IS COMPLETE AND BUILDING IS DRIED IN.

7. SEE SHEET G002 FOR PROJECT SPECIFICATION LIST. REVIEW ALL NOTES PRIOR TO CONSTRUCTION.

8. COORDINATE WITH ELECTRCIAL DRAWINGS FOR ALL LIGHTING, POWER AND DATA REQUIREMENTS.

LEVEL 1 - REFLECTED CEILING PLAN

1/4" = 1'-0"

GARAGE WEST ELEVATION

GARAGE EAST ELEVATION

Plot Date: 8/26/2019 11:03:56 AM

GARAGE SOUTH ELEVATION

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DIRECT APPLIED SYNTHETIC STUCCO SYSTEM SEE SPECIFICATIONS FOR TYPE AND BINSH STORE VENEER SE SPECIFICATIONS FOR TYPE AND BINSH STORE VENEER SE SPECIFICATIONS FOR TYPE AND BINSH ROOF ASYNALT SE SPECIFICATIONS FOR TYPE, PATTERN AND COLOR. ROOF STANDING SEAM SE SPECIFICATIONS FOR TYPE, PATTERN AND COLOR. ROOF REPERT CONTINUED FOR THE PATTERN AND COLOR. ROOF REPERT CONTINUES FOR THE PATTERN AND COLOR. ROOF REPERT FOR THE PATTERN AND SEE STURCTURAL REPERTERNENT AND AND REPERT FOR THE PATTERN AND CONTINUES AND DETAILS SERVECTERAL STELL PATTERNENT AND SEE STURCTURAL SELECTED BY OWNER. REPERTERNENT AND AND REPERT FERE PARTICLES BY OWNER. REPERTERNENT AND AND REPERT FERE REPERT PATTERNENT AND DETAILS SELECTED BY OWNER. SEE DOOR SCHEDULED AND D	DRECT APPLIED SYNTHETIC STUCCO SYSTEM SEE SPECIFICATIONS FOR TYPE AND FINSH SEE SPECIFICATIONS FOR TYPE AND FINSH SEE SPECIFICATIONS FOR TYPE AND FINSH SEE SPECIFICATIONS FOR TYPE AND FINSH FOOF ASHIAL FOOF ASHIAL FOOF ASHIAL FOOF STANDING SEA SEE SPECIFICATIONS FOR TYPE PATTERN AND COLOR FOOF STANDING SEA SEE SPECIFICATIONS FOR TYPE PATTERN AND COLOR FRE TREATED - HORIZONS FOR TYPE PATTERN AND COLOR FRE TREATED AND FRE TREATES TO THE PATTERN AND COLOR FRE TREATED AND FRE TREATES TO THE PATTERN AND COLOR FRE TREATED AND FRE TREATES TO THE PATTERN AND COLOR FRE TREATED AND FRE TREATES TO THE PATTERN AND COLOR FRE TREATED AND FRE TREATES TO THE PATTERN AND COLOR FRE TREATED AND FRE TREATES TO THE PATTERN AND COLOR FRE TREATES FRE TREATED AND FRE TREATES AS FRE TRUCTURAL DRAWINGS AND DEFAILS FRE TREATED AND FRE TREATES AS FRE TRUCTURAL DRAWINGS AND DEFAILS FRE TREATED AND FRE TREATES AS FRE BUILDING COLOR SEE ACHIFICITURAL FRE TREATED AND FRE TREATED AS FRE BUILDING COLOR SEE ACHIFICATIONES	HATCH PATTE	ELEVATION / SECTION MATERIAL LEGEND
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			TRANSISTIONS WHETHER SHOWN OR NOT.

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			STONE VENEER SEE SPECIFICATIONS FOR TYPE, LAY AND FINISH
			ROOF ASPHALT SEE SPECIFICATIONS FOR TYPE, PATTERN AND COLOR.
			ROOF STANDING SEAM SEE SPECIFICATIONS FOR TYPE. PATTERN AND COLOR
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			01-00 ALL CONSTRUCTION SHALL CONFORM TO ALL 2015 INTERNATIONAL CODE (I.R.C.), UTAH AMMENDMENTS, LOCAL, AND RELATED BUILDING AND STD. CONST. PRACTICES IN EFFECT. 01-07 THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE TO COORDINAT BETWEEN TRADES AND THAT WORK IS COMPLETED IN ACCORDACE V APPLICABLE BUILDING CODES. 01-08 CONTRACTOR SHALL REPORT ANY DISCREPANCIES IN THE PLANS TO
	LEVEL 1 - FLOOR PLAN		ARCHITECT AND ENGINEER PRIOR TO COMMENCING RELATED WOR 03-01 CAST IN PLACE FOOTINGS TO BEAR ON UNDISTURBED SOIL OR ENG. COMPACTED FILL. SEE STURCTURAL.
			03-02 CAST IN FLACE FOUNDATION WALLS TO BE STITLER AS TERFERANS W PROOFING AS NOTED. SEE STURCTURAL. 03-03 CAST IN PLACE INTERIOR CONCRETE SLABS TO BE 4" CONCRETE SLAB REINFORCED WITH FIBER MESH OVER 4" GRAVEL BASE. SEE STURCTUR. 04-01 4" EXTERIOR STONE VENEER STONE VENEER AS SELECTED BY OWNER.
			04-01 4 EXTERIOR STONE VENEER STONE VENEER AS SELECTED BT OWNER / SEE DETAILS. 05-02 STRUCTURAL STEEL BEAMS, SEE STRUCTURAL DRAWINGS AND DETAILS SELECTED.
			07-01 SPRAT APPLIED FOUNDATION WATERPROOFING: SEE ARCHITECTORA 07-02A THERMAL INSULATION SYSTEM. SEE INSULATION SCHEDULE, ARCHITECTORA 07-02B ACOUSTIC INSULATION SYSTEM. SEE INSULATION SCHEDULE, ARCHITECTORA
	LEVEL 0 - FLOOR PLAN		07-05 ROOF/FLOOR VENTILATION AS PER BUILDING CODE. SEE DETAILS. 07-06 METAL SHEET ROOFING, FLAT SEAM AS SELECTED BY OWNER/ARCHITI AS PER DETAILS.
			07-07 SQUARE RAIN GUTTER SYSTEM W/ SQUARE DOWNSPOUTS PER DETAILS 07-08 OPTIONAL SNOW CLEATS/GUARDS. SEE DETAILS. 08-06 ALUMINUM CLAD WOOD PATIO DOOR W/ DBL. INSUL LOW E GLAZIN DOOR SCHEDULE, DETAILS.
			08-07 OPERABLE DOOR AND WINDOW WALL W/ DBL. INSUL LOW E GLAZIN SCHEDULE, DETAILS. 08-08 ALUMINUM CLAD WOOD WINDOWS W/ DBL. INSUL LOW E GLAZING. SCHEDULE, DETAILS.
Section 9			12-01 CABINETRY, BUILT IN AS PER INTERIOR ELEVATIONS, DETAILS, AND INTERDESIGNER SELECTIONS. EL-01 CONTRACTOR SHALL VERIFY THAT ALL GRADE SLOPES AWAY FROM E SLOPE SHALL BE 6" SLOPE IN FIRST 10'-0".
			EL-06 PROVIDE FLASHING DIVERTER AT ALL LOCATION WHERE ROOF TERMI SIDE OF WALL. DIVERTOR SHALL BE INSTALLED TO DIRECT WATER AW WALL. EL-08 CONTRACTOR SHALL PROVIDE FLASHING AT ALL SIDING / MATERIAL
			TRANSISTIONS WHETHER SHOWN OR NOT. RF-01 ROOFING SHALL BE INSTALLED OVER CONTINUOUS BITUTHENE UNDER AND 30# SLIP SHEET AT METAL ROOF. RF-02 ROOFING ON SLOPES LESS THAN 4 AND 12 PITCH SHALL HAVE 2 CON
			LAYERS OF BITUTHENE UNDERLAYMENT PRIOR TO FINISH ROOFING. RF-05 ALL PENETRATRATION BY MECHANICAL DUCTWORK OR VENTING SHA FLASHED AS PER MANUFACTURER, CONTRACTOR TO COORDINATE. S-01 CONTRACTOR SHALL EXTEND WALL FRAMING, GYPSUM BOARD AND
			TO UNDERSIDE OF ROOF DECK. S-05 EXTEND RIGID INSULATION AT INTERIOR FACE OF CONCRETE FROM U OF SLAB TO TOP OF FOOTING. SL-7 CONTRACTOR TO COORDIANTE FOOTING STEPS TO ASSURE REQUIRE DEPENDENT AT STOLEN AT STOLEN AT INTERIOR FOOTING STEPS TO ASSURE REQUIRE
			SL-8 CONTRACTOR TO COORDINATE FOUNDATION WALL STEPS WITH FINA SPECIFIED.
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		STONE VENEER SEE SPECIFICATIONS FOR TYPE, LAY AND FINISH
		ROOF ASPHALT SEE SPECIFICATIONS FOR TYPE, PATTERN AND COLOR.
		ROOF STANDING SEAM SEE SPECIFICATIONS FOR TYPE. PATTERN AND COLOR
		FIRE TREATED - HORIZONTAL CEDAR SIDING SEE SPECIFICATIONS FOR TYPE. PATTERN AND COLOR
		NOTE: REFER TO MATERIAL SPECIFICATIONS DOCUMENT FOR DETAILED INFORMATION REGARDING EACH FINISH MATERIAL
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A601

3" = 1'-0"

NUMBER	TYPE	COUNT	WIDTH	HEIGHT	FRAME TYPE	FIRE RATING	DETAIL HEAD	DET
1		,	4.01				(14.00)	
1	DI		4-0*	8-0	FI		6/A602	9
2	D2	1	2'-4"	8'-0"	F1		5/A602	14
2	D2	7	2'-6"	8'-0''	F1		5/A602	14
2	D2	1	2'-10"	8'-0''	F1		5/A602	14
3	D2	2	3'-0"	8'-0''	F1		5/A602	14
4	D3	2	3'-0"	8'-0''	F2	2 HR	VARIES	V
5	D2	5	2'-10"	8'-0''	F1		5/A602	1,
6	D4	4	2'-0"	7'-0''	-		-	
7	D5	4	6'-0"	8'-0''	F1		5/A602	1,
8	D6	2	9'-0"	9'-0''	F3	NON-RATED	4/A602	7
9	D2	1	2'-4"	8'-0''	F1		5/A602	1.

TYPICAL DOOR AS PER PLANS - SEE

- LOCATED DOOR 6" FROM WALL TYPICAL

DOOR SCHEDULE.

A - - - - - - A

1 1/2" = 1'-0"

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A603

WINDOW W12

WINDOW W13

2'-6"

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fax 801.269.1425 fax 801.269.1425 www.thinkaec.com The designs shown and described herein including all technical drawings, graphic representation & models thereof, are proprietary & can not be copied, duplicated, or commercially exploited in whole or in part without the sole and express written permission from THINK Architecture, inc.

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WINDOW W17 1/2" = 1'-0"

WINDOW W18 1/2" = 1'-0" 4 A604

WINDOW W19 1/2" = 1'-0"

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GSN - General

- 1 These drawings have been prepared by the Engineer of Record primarily to safeguard against major structural damage and loss of life, not to limit damage or maintain use of the structure. See the requirements of the current accepted building code, and as listed in the Basis for Design.
- 2 Professional standards of care have been used in the preparation of these drawings, normally
- exercised under similar circumstances by reputable engineers in this, or similar localities. 3 Design of non-structural elements (i.e. stairs, railings, non-load bearing walls, veneers, curtain walls,
- etc.) and their attachment is not included, and must be provided by others, unless specifically noted on these drawings. 4 Design of pre-engineered structural products (i.e. wood trusses, steel joists, or pre-cast concrete
- elements, etc.) is not included and must be provided by others, unless specifically noted on these drawings. 5 Specification references (i.e. ASTM, ACI, AWS, etc.) shall be the latest accepted version, where noted
- on these drawings 6 An experienced, licensed contractor, with a working knowledge of applicable codes and industry
- accepted standard practices, shall perform the work depicted in these drawings. 7 All work shall conform to the minimum standards of the current accepted building code found in the Basis for Design, other codes, industry specific specifications, and standards listed herein. The contractor shall comply with requirements of all regulatory agencies with authority over any portion of the work. Work not specifically shown on these drawings shall conform to all applicable codes and accepted standard practices.
- 8 The contractor shall verify all dimensions, elevations, and conditions on these drawings with the architectural drawings, and all other discipline drawings, prior to start of construction. Notify the Engineer of Record in writing before the start of construction regarding discrepancies, omissions, or variations, or they will become the sole responsibility of the contractor. Notes and the specific details on these drawings take precedence over the Structural General Notes, and General Details.
- 9 Construction methods are not indicated on these drawings. The contractor shall be solely responsible for all methods, sequences, and procedures of construction. The contractor shall provide adequate shoring, bracing, form work, etc. as required for the protection of life and property during construction.
- 10 Excavation procedures, including shoring and protection of adjacent property, structures, streets and utilities, shall be performed in compliance with local building codes, regulations and safety requirement, and shall be the contractor's responsibility.
- 11 Construction materials shall be spread out uniformly on structural systems, such that design live loads are not exceeded.
- 12 Structural members shall not have openings, pockets, etc. larger than 6" placed in them, unless specifically noted on these drawings. When drawings by others show items in structural members not shown in the structural drawings, the Engineer of Record shall be notified in writing to determine the
- appropriate solution. 13 Visits to the construction site by the Engineer of Record are a resource for the contractor and shall not be considered as special inspection.

		GSN - Basis fo	or Design	
1	Governing Building			2015 International Building
2	RoofLoads			0000
-	11001 200.00	Pitched Roof		
			Dead Load	= 25 psf
			Live Load	= 20 psf (reducible)
3	Floor Loads			
			Dead Load	= 40 psf
			Live Load	= 40 psf (Residential)
4	Snow Loads			
		Snow Loads		
			Ground	= 235 psf (elevation)
			Flat Roof	= 165(psf)
5	Wind Design			
		Basic Wind Speed		= 115 mph
		Wind Exposure		= C
		Importance Factor		= 1.0
		Mean Roof Height		= 25 ft.
6	Seismic Design	I		
		Design Catgegory		= D
		Use Group		= 1
		Sds		= 0.587
		Sd1		= 0.310
		Site Class		= D
		Lateral Force Reisting System		= Wood Shear Walls
		R		= 6.5
		Analysis Procedure		= Equivalent Method
		Base Shear Coefficient		= 0.065 x W

GSN - Foundations

- 1 Foundations were designed according to the minimum requirements of the current accepted building code as listed in the Basis for Design. A Geotechnical Engineer should be commission to provide a soils report prior to the completion of the structural design for this project. Sive Engineering will not assume any liability beyond the minimum code requirements in the event that a Geotechnical Report is not provided.
- 2 Footings & Foundations:
- Allowable Soils Pressure (eq. fluid weight) = 1500 psf 3 Cantilevered Retaining Walls:
- Active Soil Pressure (eq. fluid pressure) = 35 psf
- Passive Soil Pressure (eq. fluid pressure) = 250 psf Sliding Resistance (friction coefficient): = 0.35
- 4 Restrained Retaining Walls:
- Active Soil Pressure (eq. fluid pressure) = 50 psf
- 5 Do not backfill against foundation walls more than 3'-0" in height until after the top of the concrete wall is braced by the completed interior floor systems and all elements have reached their design strength. 6 All forms shall be properly braced to withstand the placement of fresh concrete.
- 7 Footing excavations shall be clean and free from loose debris, standing water, or un-compacted material at the time of concrete placement.
- 8 Trenches and excavations under or adjacent to foundations or slabs shall be properly backfilled and compacted. Utility trenching parallel to the foundation shall be located a minimum distance equal to the depth of the trench from the foundation. The trench may approach the foundation at 90 degrees to the structure and may not exceed two and one half feet wide. The trench approach to the foundation may not be located closer than 8 feet from a corner of the structure.

GSN - Concrete

- 1 Compressive strength, f'c, shall be 4500 psi and a maximum water/cement ratio of 0.45 for concrete in contact with soil. All other structural concrete f'c shall be 3000 psi. Foundation design uses 2500 psi, therefore, special inspection is not required. 2 Concrete mixes shall be designed by a certified concrete testing laboratory and approved by the
- engineer of record.
- 3 All concrete shall be normal weight 145 pcf with hard-rock aggregates. 4 Maximum slump shall be 5 inches, and the water shall be clean and potable.
- 5 Portland cement shall be ASTM C 150 type V for concrete in contact with soil. Type II cement may be used elsewhere. All cement in contact with soil shall comply to the table above regarding sulfate exposure.
- 6 Fly ash shall comply with ASTM C 618, class F, and shall be approved by the architect in writing prior to being used on the job. When used, fly ash content shall be 15%-25%. Water-cement ratio shall be based on total cementitious material.
- 7 Aggregates shall comply with ASTM C 33. Use 3/4 inch maximum aggregate in structural concrete. 1-1/2 inch maximum in slabs on grade and 3/8 inch pea gravel in grouts, unless specifically noted
- otherwise on the plans, or by written approval of the engineer of record.
- 8 No more than 90 minutes shall elapse between concrete batching and placement of concrete unless approved in writing by the engineer of record. 9 Concrete mixing, placement and quality shall be per the current accepted code (listed in the basis for design). Mechanically vibrate all concrete. Vibrate slabs on grade around and under floor ducts or
- similar elements. 10 Control joints in slabs on grade shall be as noted in the general details. Saw-cut joints shall be cut to a minimum depth of t/4. Doweled joints shall be used where noted on plans. Do not joint post-tensioned concrete slabs on grade unless noted otherwise on plans. Space control joints as listed below:

concrete staps on grade unless noted otherwise on plans. Space				
	Slab Thickness (t)	Joint spacing (each way)		
	4"	10'-0"		
	5"	12'-6"		
	6"	15'-0"		

concrete slabs.

- 11 Remove all debris from forms before placing concrete. Concrete shall be carefully placed in reinforced elements to avoid segregation of aggregates. Unconfined fall of concrete shall not exceed five feet, unless approved in writing by the engineer of record.
- 12 Reinforcing, dowels, bolts, anchors, sleeves, embeds, etc. shall be securely positioned in the forms prior to placement of concrete. 13 High early strength concrete may be used when requested by the contractor. Mix design data using
- field cured specimens shall be submitted for review and approval. 14 Protect concrete from damage or reduced strength due to cold or hot weather in accordance with ACI 305 and 306. Contractor shall take special curing precautions to minimize shrinkage cracking of

GSN - Reinforcing Steel

- Reinforcing steel shall meet ASTM A615 and shall be grade 60 deformed bars for all bars #5 and larger. Reinforcing may be grade 40 deformed bars for all bars #4 and smaller. All reinforcing to be welded shall be ASTM A706, grade 60 low alloy weld-able steel.
- 2 Welded wire fabric shall meet ASTM A185. Lap all welded wire fabric at least one row of wires plus 2 inches.
- 3 All reinforcing steel dimensions are center to center of the steel unless noted as clear (CLR) cover. Minimum cover for reinforcing shall be as follows (unless noted otherwise on the plans): Exposure
- Cast against and permanently exposed to earth
- Exposed to earth or weather #5 bar and smaller
- #6 bar and larger
- Not exposed to earth or weather Slabs, walls and joists
- #11 and smaller
- #14-#18 Beams and columns
- Primary reinforcing, ties Stirrups, and spirals
- Slabs on grade 4 Lap splices in beams, slabs and footings shall be per current governing code or lap schedule where present. Stagger splices a minimum of one lap length. The tack welding of reinforcing bars shall not be allowed. Provide bent corner bars to match and lap with horizontal bars at all corners and intersections per general details. Vertical wall bars shall be spliced at or near floor lines. Splice top bars at center line of span and bottom bars at the support in spandrels, beams, grade beam, etc. unless
- noted otherwise on the plans. 5 Mechanical splice couplers shall have current testing report accepted by local building official and
- shall be capable of developing 125% of the strength of the bar. 6 All reinforcing shall be bent cold, one time only. Field bending of rebar shall not be allowed unless
- specifically noted on the plans. 7 Welding of reinforcing bars, metal inserts, and connections shall conform to AWS D1.4, and shall be
- made only at locations shown on plans or details. 8 All welds involving reinforcing bars shall be an E90 low hydrogen electrode.
- 9 Reinforcing bar spacing shown on the plans represents the maximum on center spacing. All bars shall be detailed and placed per the current governing code as indicated in the basis of design.
- 10 Dowel all vertical reinforcing to foundation, as specified on plans or details. Securely tie all bars in location prior to placement of the concrete.
- 11 Minimum clear spacing between parallel reinforcement shall be 1 1/2 times bar diameter, 1 1/2 times
- the max aggregate size, or $1 \frac{1}{2}$ " (whichever is larger).

	GSN - Lap Splice Schedule															
	f'c=3000 psi			f'c=4000 psi f'c=5000 psi			f'c=6000 psi									
Bar	Regular		Regular Top	Reg	ular	To	р	Reg	Regular Top	Regular	Tc	Тор				
Size	Class		Cla	ass	Cla	ass	Cla	ass	Cla	class Class		ass	Class		Cla	ass
	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В
#3	13"	17"	17"	21"	12"	16"	16"	21"	12"	16"	16"	21"	12"	16"	16"	21"
#4	17"	22"	22"	28"	15"	19"	19"	25"	13"	17"	17"	22"	12"	16"	16"	21"
#5	21"	27"	27"	35"	18"	24"	24"	31"	16"	21"	21"	27"	15"	19"	19"	25"
#6	27"	36"	36"	46"	24"	31"	31"	40"	21"	28"	28"	36"	20"	25"	25"	33"
#7	37"	48"	48"	63"	32"	42"	42"	54"	29"	38"	38"	49"	27"	34"	34"	44"
#8	49"	64"	64"	82"	42"	55"	55"	71"	38"	49"	49"	64"	35"	45"	45"	58"
#9	62"	80"	80"	104"	54"	70"	70"	90"	48"	62"	62"	81"	44"	57"	57"	74"
#10	78"	102"	102"	132"	68"	88"	88"	115"	61"	79"	79"	102"	56"	72"	72"	94"
#11	96"	125"	125"	162"	83"	108"	108"	141"	76"	97"	97"	126"	68"	88"	88"	115"
			GS	N -	Lap	o Sp	olic	e S	che	edu	le l	Vot	es			

. . These notes shall be used for all splices, unless noted otherwise on plan

- 2 Class "A" splices may be used only in cases where 50% or less of the bars are spliced within the lap splice length
- 3 Class "B" splices may be used for all splices unless the requirements of note 2 are met
- 4 Ties & stirrups shall not be spliced 5 a. For bundles bars or three or less, lap splice length shall be multiplies by 12
- b. For bundled bars of four or more, lap splice lengths shall be multiplied by 133 c. Indiviualn bar splices within a bundle shall not overlap
- d. Entire bundles shall not be lap spliced 6 For all lightweight concrete, lap lengths shall be multiplied by 13
- 7 for all epoxy coated bars, lap lengths shall be multiplied by 13 for top bars and 15 for bottom bars
- reinforcing bar

GSN - Anchor Bolts

- 1 Sill plate anchorage at concrete or masonry shall be 1/2" diameter embedded anchor bolts @ 36" o.c. (U.N.O.). All anchor bolts (excluding bolts for holdowns) shall be embedded 7" minimum into the concrete. Anchor bolts for holdowns shall not be considered as part of the required sill plate anchor
- bolts, as specified in the shearwall schedule or Structural General Notes. Interior walls may be anchored to the concrete with the following (unless noted otherwise on plans):
- Embedded anchor bolts
- Concrete screw anchors Expansion anchors
- Powder driven fasteners (as specified below)
- 2 At all sill plates there shall be a minimum of two bolts per piece of plate with one bolt located not more than 12" or less than seven bolt diameters from each end of the piece of plate. A properly sized nut and washer shall be tightened on each bolt to the plate. For seismic design categories D-F, washers shall be a minimum or 0.229" thick by 3" square plate washer (refer to "Basis for Design" on this sheet for seismic design category) at exterior walls, shearwalls, and interior bearing walls where 2x sill plates are specified. Foundation plates and sills shall be the kind of wood specified in the current approved code as listed in the Basis for Design.
- 3 All shearwalls shall be anchored to the concrete per the shearwall schedule 4 Where exterior wall embedded anchor bolts have been missed, damaged, or improperly located, one of the following retro-fit options may be used at the contractors discretion: 4.1 Provide (1) 1/2" diameter Simpson Titen Screw Anchors (ESR-1056 & ESR-2713) concrete screw
- anchor. Concrete screw anchors shall be embedded a minimum of 4 1/2". 4.2 Epoxy bolts of the same diameter and spacing may be used in lieu of the embedded bolts. A 7" minimum embedment shall be provided for epoxy grouted bolts. 5 Where interior wall embedded anchor bolts have been missed, damaged, or improperly located, one
- defined as anchor bolts that are located a minimum of 6" from slab edges, steps, turn-downs, openings, or similar discontinuities: 5.1 Provide (1) 1/2" diameter Simpson Titen Screw Anchors (ESR-1056) or 1/2" diameter ITW
- Ramset/RedHead LDT (ER 5890) concrete screw anchor. Concrete screw anchors shall be embedded a minimum of 4 1/2"

5.2 Epoxy bolts of the same diameter and spacing may be used in lieu of the embedded bolts. A 7" minimum embedment shall be provided for epoxy grouted bolts. 5.3 Shot pins may be used (either Ramset 3500 series (ESR-1799) or Hilti (ESR-1663)) or any other ICC-ES approved powder driven fastener with at least 200# allowable shear resistance in 2000 psi concrete per the schedule below. Install all fasteneres per ICC-ES Evaluation Report and manufacturer's recommendations. Embedment shall be 1 1/4" minimum and the shot pins shall be installed such that the sill plate wood does not split. If splitting occurs, equivalent expansion bolts shall

e installed per the requirements above.			
	Sh	not Pins	
Required anchor bolt spacing	0.170 dia.	0.140 dia.	
1/2" dia. bolts @ 72" o.c.	13"	10"	
1/2" dia. bolts @ 60" o.c.	11"	8"	
1/2" dia. bolts @ 48" o.c.	9"	6"	
1/2" dia. bolts @ 40" o.c.	7"		
1/2" dia. bolts @ 36" o.c.	6"		
5/8" dia. bolts @ 72" o.c.	8 1/2"	6"	
5/8" dia. bolts @ 60" o.c.	7"		
5/8" dia. bolts @ 48" o.c.	5 1/2"		
3/4" dia. bolts @ 72" o.c.	6"		

6 Interior non-load bearing partition walls may be anchored to the slab with a minimum 0.140" diameter shot pins at 32" o.c. maximum spacing.

GSN - General Structural Notes

Min. Cover Tolerance

3"	+/- 3/8"
1 1/2"	+/-3/8"
2"	+/-3/8"
3/4"	+/- 3/8"
1 1/2"	+/- 3/8"
1 1/2"	+/- 3/8"

1 1/2" +/- 3/8

e Schedule	

8 Top bars are classified as horizontal bars where 12", or more, of fresh concrete is cast below the

of the following retro-fit options may be used at the contractors discretion. Interior anchor bolts shall be

GS	N - Wood
1 Structural sawn lumber design values shall Western Wood Products Association (WWP sawn lumber shall be stamped with the gra Structural sawn lumber components shall h on plans):	comply with the latest edition of the grading rules of the PA) or the West Coast Lumber Inspection Bureau (WCLIB). All ade match of an approved lumber grading agency. have the following minimum grade (unless noted otherwise
. Use	Material
. 2x sill plates	Treated Douglas-Fir
. 2x top plates	Douglas Fir Stud Grade
. 2x4 studs/blocking	Douglas Fir Stud Grade
. 2x6 studs (up to 10'-0" in height)	Douglas Fir Stud Grade
. 2x6 studs (over 10'-0" in height)	Douglal Fir No. 2
. Joists and all other sawn lumber	Douglas Fir No. 2
. 6x beams and 6x post	Douglas fir No. 1
2 Glue-Laminated beams (GLB)shall be Dou cantilevered GLB beams shall be Douglas properties:	glas Fir 24F-V4 unless noted otherwise on the plans. All Fir 24F-V8. The GLB beams shall have the following minimum
. E=1,800,000 psi	
Eb. 2400 pci	

Fb=2400 psi Fv=165 psi

Fabrication and handling shall conform to the latest AITC and ASTM standards. Beams shall bear an apporpriate grade stamp clearly noting its design properties. Beams shall be manufactured with industry standard minimum camber (2000' radius) unless camber is specifically noted on the plans. Fabrication and handling shall conform to the latest AITC and ASTM standards. Beams shall bear an apporpriate grade stamp clearly noting its design properties. Beams shall be manufactured with industry standard minimum camber (2000' radius) unless camber is specifically noted on the plans. 3 Laminated Veneer Lumber (LVL) shall be Douglas fir and manufactured in accordance with TrusJoist Macmillan Corporation manufacturing standards as referenced in NER-481, or approved equal. All LVL

- members shall have the following minimum properties: E=1,800,000 psi
- Fb=2600 psi Fv-285 psi
- Fc(parallel)=2510 psi Fc(perpendicular)=750 psi

When multiple LVL pieces are grouped together, they shall be fastened with (2) rows of 16d nails at 12" o.c. for member depths up to 14" in depth. LVL members greater than 14" in depth shall be used in built up sections only, and shall be fastened together with (3) rows of 16d nails at 12" o.c. 4 All sills or plates resting on concrete or masonry shall be pressure treated Douglas Fir or other locally

- approved chemically treated lumber. 5 All beams shall be considered flush bottom unless noted otherwise on the plans. Girder trusses and beams shall have full bearing (for example a (3) ply girder truss shall have a minimum of (3) 2x studs carried to the foundation or carrying beam per plans) at each bearing point with (2)2x studs minimum. Nail studs together per typical nailing schedule. Beams and girder trusses ((2)ply or larger) bearing on the top plate shall be attached to the top plate with an A34 framing anchor or (2) 16d toenails each side of the structural member (unless noted otherwise on the plans).
- 6 Openings in a single level or top level of the structure shall be framed as follows Widths less than 6'-0" wide shall be framed with (1)2x king stud and (1) 2x trimmer stud each side of
- the opening (unless noted otherwise on the plans). Widths greater than 6'-0" but less than 16'-0" shall have (2) 2x king studs and (1) 2x trimmer stud each side of the opening (unless noted otherwise on the plans). 7 In exterior walls, interior bearing partitions and shear walls, any wood stud may be cut or notched to a
- depth not exceeding 25% of its width. Cutting or notching of the studs to a depth greater than 40% of the width of the stud is permitted in non-bearing partitions supporting no loads other than the weight of the partition. The cut or notched stud shall be mechanically reinforced per the general detail.
- 8 A hole not greater in diameter than 40% of the stud width may be bored in any wood stud. Bored holes not greater than 60% of the width of the stud is permitted in non-bearing partitions or in any wall where each bored stud is doubled, provided not more than two such successive doubled studs are bored. In no case shall the edge of the bored hole be nearer than 5/8" to the edge of the stud. Bored holes shall not be located at the same section of stud as a cut or notch. Do not notch joists, rafters or beams, unless noted othewise on the plans. Approval for any holes or notches not indicated on the plans shall be provided by the engineer of record, in writing, prior to the work being done on the site
- 9 All bolt shall be installed in holes bored with a bit 1/16" larger than the diameter of the bolt. Bolts and nuts seating on wood shall have cut steel washers under heads and nuts. Ding threads to prevent loosening. Lag bolts shall be installed in pre-drilled holes by hand turning with a wrench (not with an electric or pneumatic impact tool).
- 10 All nails (except 16d nails) shall be common nails unless specifically noted otherwise on the plans. 16d nails may be 16d sinker, 16d box, pneumatic (P-nail), or 12d common, unless noted otherwise on the plans. Nails shall be driven so that heads are flush with wood surface. Over or under driven nails will not be acceptable. Miscellaneous nailing shall be per the current approved code nailing schedule, or as listed below:

Nail Size	Shank Diameter	Lengt
16d common	.162"	3 1/2"
16d sinker	.148"	3 1/4"
16d box	.135"	3 1/2"
12d common	.148"	3 1/4"
10d	.148"	3"
8d	.131"	2 1/2"

11 A.P.A. performance rated sheathing (O.S.B.) may be used as an alternate to plywood with prior approval of owner and/or architect. Rated sheathing shall comply with ICC-ES Evaluation Report No. ESR-2586, exposure 1, and shall have a span rating equivalent to or better than the plywood it replaces. Install per manufacturer's recommendations.

12 Shear panel blocking noted on plans or details shall be constructed of 2x solid framing with 3/8" minimum plywood with 8d nails at 6" o.c. (unless noted otherwise on plans) and shall be nailed to

adjacent trusses/joists with minimum (2) 16d at the top and bottom of truss or framing. 13 All plywood laid with face grain perpendicular to supports shall be C-D or C-C sheathing conforming to current adopted code as listed in the Basis for Design and shall coform to the following nominal thickness, span rating, and nailing pattern below (unless noted otherwise on the plans):

	0.		
	Nai	ling Pattern	
Thickness	Span Rating	Edge Nailing	Field Nailing
3/8"	24/0	8d@6" o.c.	8d@12" o.c.
7/16"	24/16	8d@6" o.c.	8d@12" o.c.
15/32"	32/16	8d@6" o.c.	8d@12" o.c.
3/4"	48/24	10d@6" o.c.	10d@12" o.c.
1"	60/48	10d@6" o.c.	10d@12" o.c.
1 1/8"	48" O.C.	10d@6" o.c.	10d@12" o.c.

GSN - Equivalent Spacing of Approved Fasteners

	Specing	Staples & Nails Gauge				
Nali size	spacing	16	15	14		
6d	4"	3 1/2"	4"	5"		
6d	6"	5"	6"	7"		
6d	8"	6 1/2"	8"	9 1/2"		
6d	10"	8 1/2"	10"	12"		
6d	12"	10"	12"	14 1/2"		
8d	4"	2 1/2"	3 1/2"	4"		
8d	6"	4"	5"	6"		
8d	8"	5 1/2"	6 1/2"	8"		
8d	10"	6 1/2"	8"	10"		
8d	12"	8"	10"	12"		
10d	4"	2"	2 1/2"	3"		
10d	6"	3 1/2"	4"	5"		
10d	8"	4 1/2"	5 1/2"	6 1/2"		
10d	10"	5 1/2"	7"	8"		
10d	12"	6 1/2"	8"	9 1/2"		

	and leg
5	Trusses r
	panel e
6	The truss
	required
7	Any pro
	drawing
	docume
Late	eral Ties @
	Plate
	H1

the plans.

GSN - Nailing Schedule						
Types of Connections	Minimum Nailing					
Joists or truss top plate, sill, etc.	(3)16d, toenail					
Bridging to joist	(2)8d, toenail each end					
Sole plate to joist or blocking	16d @ 16" o.c.					
Top plate to stud	(2) 16d, end nail					
Stud to sole plate	(2)16d end nail or Simpson A34					
Double or muliple built-up studs	16d @ 16" o.c. staggered, each piece					
Double or multiple top plates	16d @ 16" o.c. staggered, each piece					
Blocking between joists or rafters to top plate	(3)16d, toenail					
Rim joist to top plate	16d @ 16" o.c., toenail					
Top plates, laps and intersections	(2)16d					
Continuous header, two or more laminations	16d @ 16" o.c. along each edge, each piece					
Ceiling joists to top plate	(3)16d, toenail					
Continuous header to stud	(3)16d, toenail					
Ceiling joists, laps over partitions	(3)16d					
Ceiling joists to parallel rafters	(3)16d					
Rafter to truss to plate	(3)16d, toenail					
Built-up corner studs	16d @ 16" o.c. staggered, each piece					
Built-up channel blocking @ wall intersecting shear wall	16d @ 6" o.c., each piece, up to SW4					
Built-up channel blocking @ wall intersecting shear wall	16d @ 3" o.c., each piece, up to SW8					
King stud to beam (end)	(2)16d @ 3" o.c. (4 min.)					

GSN - Nailing Schedule Notes

A Minimum nailing per this schedule unless noted otherwise on the plans or details B All nailing is face nailing unless noted otherwise on plans or details

C See general details for top plate splice connection D See IBC table 2304.9.1 for conditions not listed

galvanized steel.

Simpson

Hold Down

HDU4

HDU5

HDU8 HDU11

HDU14 HDQ8

HHDQ14

HDC10

GSN - Wood Connection Hardware

1 Manufactured hardware shall be Simpson Strong-tie products as noted on the drawings. Alternatives may be used at the discretion of the Engineer of Record. One manufacturer shall be used exclusively for each project. Only approved connectors listed below shall be used: a) Simpson Strong-Tie Company Inc, Brea California, (ICC-ES ESR report No.1866, 2203, 1622, 2105, 2236, 2330, 2549, 2551, 2552, 2553, 2554, 2604, 2605, 2606, 2607, 2608, 2613, 1614, 2615, 2616, 2877, 2555,

2611, 3046, 2920). b) USP Lumber Connectors, 2150 Kitty Hawk Road, Livermore, California 94550, (ICC-ES ESR report No. 1178, 1280, 1702, 1781, 1881, 1970, 2104, 2685, 1831, 1465, 2761, 2787).

2 Tension hold down anchors and straps shall be installed as specified on the foundation and the roof framing plans. The following equivalent hold down anchors and straps substitutions may be used at the contractors discretion: Stra old Down

ap Tie Hold Down	Bolt Type Ho
LSTHD8	HDU2
STHD8	HDU2
STHD10	HDU2
LSTHD8	HDU2
a a hara may ba a nahara a	l ta aanarata dab with ar

3 Hold down anchors may be anchored to concrete slab with epoxy-grouted all-thread bar in drilled hole. Alternate epoxy systems may be used provided ICC-ES reports are submitted and the alternate system have been approved by the EOR. Install per ICC-ES report and manufacturer's specifications. Special inspection is required during all epoxy anchored bolt installations.

4 Adhesives used to attach floor sheathing to the framing shall conform to the AFG-01 specification of the American Plywood Association (adhesives for field-gluing plywood to wood framing). The adhesive shall be certified as conforming to AFG-01 by a testing agency approved by the building official or accepted by the federal housing administration. Alternates may be used only with specific approval of the EOR and only upon submittal of a listing of adhesives to be substituted. 5 Where the truss manufacturer requires additional bearing an appropriately sized Truss Bearing Enhancer (TBE) connection shall be installed per the hardware manufacturer specifications 6 Fasteners for preservative-treated and fire-retardant-treated wood shall be of hot dipped zinc-coated

GSN - Anchor Bolt Retro-Fit							
Minimum	Mimimum	All-Thread	Minimum Epoxy	Simpsor	ר SSTB		
Bolt Diameter	Embedment	Diameter	Grout Embedment	Mono-pour	Two-pour		
5/8"	7"	5/8"	8"	SSTB24			
5/8"	7"	5/8"	10"				
5/8"	9"	5/8"	10"				
7/8"	10"	7/8"	12"				
1"	12"	1"	14"				
1"	12"	1"	16"				
7/8"	12"	7/8"	14"				
1"	12"	1"	14"				
1"	12"	1"	16"				
7/8"	12"	7/8"	14"				

GSN - Wood Trusses

1 Prefabricated wood trusses shall be designed to support their self-weight, plus live load and superimposed dead loads including, but not limited to all mechanical and other equipment, and shall be designed to resist all drag forces, shear wall uplift and downward loads, and other special loads noted on the drawings or calculations.

2 The truss manufacturer shall design for span/240 total load deflection and span/360 live load deflection. The truss manufacturer shall account for differences between deflections of adjacent trusses at transitions in the framing system, with trusses designed to minimize differential deflection. 3 Bridging size and spacing shall be as designated by the truss manufacturer unless noted otherwise on

4 The contractor shall submit shop drawings, erection drawings and design calculations sealed by an appropriately registered engineer. Shop drawings shall show any special details required at bearing points. All connectors shall have current ICC-ES approval. A minimum of (4) sets of truss submittals shall be provided for review, with one set to be retained by the EOR. All information submitted is to be clear yible. Shop drawings or truss layout drawings shall show all loads included in the truss designs. may be over spanned to accommodate 30"x30" attic access provided that the roof sheathing edges are blocked and edge nailed at over span. s manufacturer shall be responsible for specifying all hangers or other connection hardware

d for truss installation. posed changes to the truss layout, connections, or design loads depicted on the EOR gs must be approved in writing by the EOR prior to the submittal of truss construction ents for review.

G	GSN - Hardware for Truss Uplift Forces				
	Douglas Fir-Larch		Spruc	Spruce Pine Fir	
lop	Southern Pine				
	Vertical Loads	Horizontal Loads	Vertical Loads	HUHZUHLAI LUAUS	
	165#	455#	140#	400#	
	110#	480#	110#	520#	
		720#		620#	
		1000#		860#	
		1260#		1085#	
		1450#		1245#	
		2050#		1785#	

GSN - Structural Steel

Structural steel members shall cor	form to the following standards an otherwise:	d material properties unless noted
Shape	Standard	Yield (fy)
Standard steel shapes	ASTM A36	36 ksi
Rolled wide flange sections	ASTM A572	50 ksi
Bars and Plates	ASTM A36	36 ksi
Pipes	ASTM A53	36 ksi
Tubes	ASTM A500 Grade b	46 ksi
High strength bolts	ASTM A325	
Structural steel shall be fabricated a fabrication and erection of structur	and erected in accordance with A al steel buildings.	ISC specifications for the design

or the design 3 Where a steel beam is used in connection to wood framing, a 3x-DF-L stud grade plate will be bolted to the top flange with a 1/2" diameter bolts at 24" o.c. staggered. Where a steel column is located adjacent to wood framing the steel column shall be connected to the wood framing with 1/2" diameter threaded studs at 24" O.c. All threaded rod, threaded studs, foundation anchor bolts, and all bolted connections involving wood members shall be ASTM A-307 unless noted otherwise. 4 Welders shall be AWS certified. All welding shall use E70 series low hydrogen electrodes. All welding shall conform to the latest American Welding Society standards; welds on drawings are shown as shop welds.

- Contractor may shop weld or field weld at his discretion. All full penetration welds shall be tested and certified by an independent testing laboratory.
- 5 All bolts shall be installed as bearing-type connections with threads excluded from shear plane (type "x" connection), UNO. High-strength bolts shall be snug tightened using any AISC approved method and do not require special inspections unless noted otherwise. All bolts in slotted or oversize holes and all high strength bolts shall be installed with washers.
- 6 All expansion or epoxy bolts shall have current ICC-ES/ICC rating for material into which installation occurs. Headed studs shall conform to all requirements of the latest edition of the "recommended practices for stud welding" and the "structural welding code" published by AWS. All bolts, anchor bolts, expansion bolts, etc. shall be installed with steel washers at face of wood.
- 7 Grout beneath column bases or bearing plates shall be 5000 PSI minimum non-shrink flow-able grout or dry-pack. Install grout under bearing plates before framing member is installed. At columns, install grout under base plates after column has been plumbed but prior to floor or roof installation. Grout depth shall be sufficient to allow grout or dry pack to be placed beneath plate without voids.

GSN - Deferred Submittals

- 1 Prefabricated components, specialty items and design-build elements noted on the drawings, but which require the manufacturer or supplier to provide the design, shall be submitted to the Architect and/or the EOR for review as deferred submittals. Deferred submittals required by the EOR shall include, but not be limited to, the following:
- Wood trusses Engineered wood joists and beams 2 Deferred submittals shall include calculations and drawings prepared and stamped by an appropriately licensed engineer(specialty engineer) showing the location and magnitude of loads, configuration and
- size of members and compatibility of submittal items with the primary structural system. 3 The purpose of the EOR's review shall be limited to determining that the drawings and calculations have been properly sealed, that the load criteria are in general conformance with the structural drawings and with the current accepted building code as listed in the basis for design, that connections to the primary structure are compatible with the primary design, and that the primary structure is capable of supporting the imposed loads.
- 4 The EOR will rely upon the specialty engineer's seal as certification that the deferred submittal items designed by the specialty engineer comply with the criteria set forth in the structural drawings and applicable codes and standards. The EOR will not be responsible for the adequacy of designs provided by others. 5 Allow five (5) working days for the EOR's review. One copy of each submittal will be retained for the

EOR's records.

	GSN - Special Inspections (Concrete	e) 2015	5
	Type	Continuous Special	Periodic Special
	Туре	inspection	inspection
1.	Inspect reinforcement, including prestressing tendons, and verify placement	-	Х
2.	Reinforcing bar welding:		
	a. Verify weldability of reinorcing bars other than ASTM 706;	-	х
	b. Inspect single-pass fillet welds, maximum 5/16"; and	-	Х
	c. Inspect all other welds	х	-
3.	Inspect anchors cast in concrete.	-	Х
4.	Inspect anchors post-installed in hardened concrete members.		
	a. Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustaned tension loads.	х	-
	b. Mechanical anchors and adhesive anchors not defined in 4.a.	-	Х
5.	Verify use of required deisgn mix.	-	Х
6.	Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	х	-
7.	Inspect concrete and shotcrete placement for proper application techniques.	х	-
12.	Inspect formwork for shape, location and dimensions of the concrete member being formed.	-	Х

	GSN - General	Abbre	viations
ACI	American Concrete Institute	Min	Minimum
AISC	American Institute of Steel Construction	Misc	Miscellaneous
AISI	American Iron & Steel Institute	NTS	Not to Scale
Alt	Alternate	OC	On Center
ASTM	Amerian Society for Testing & Materials	plf	Pounds per Linear Foot
Cont	Continuous	psf	Pounds per Square Foot
Dia	Diameter	psi	Pounds per Square Inch
Eq	Equal	GSN	General Structural Notes
EW	Each Way	Sim	Similar
FF	Finished Floor	Spec	Specification
ft	Foot	Std	Standard
Ga	Gauge	T&G	Tongue & Groove
GLB	Glu-lam Beam	TO	Top of
Horiz	Horizontal	TOD	Top of Deck
HSS	Hollow Steel Section	TOF	Top of Footing
IBC	International Building Code	TOJ	Top of Joist
k	Kips (1000 lbs)	TOL	Top of Ledger
ksi	Kips per Square Inch	TOM	Top of Masonry
lbs	Pounds	TOS	Top of Steel
Mfr	Manufacturer	TOW	Top of Wall
Max	Maximum	Тур	Typical
Min	Minimum	UBC	Uniform Building Code
		UNO	Unless Noted Otherwise
		Vert	Vertical

GSN - Sheet Index					
Sheet Number	Sheet Name	Sheet Issue Date	Current Revision	Currer Des	
S101	General Structural Notes	9/4/18			
S102	General Structural Notes (cont.)	9/4/18	1	Correc	
S103	General Details	9/4/18			
S104	General Details (cont.)	9/4/18			
S105	General Details (cont.)	9/4/18			
S201	Basement Footing & Foundation Plan	9/4/18			
S202	Main Level Footing & Foundation Plan	9/4/18			
S301	Footing & Foundation Details	9/4/18			
S401	Basement Floor Framing	9/4/18	1	Correc	
S402	Roof Framing Plan	9/4/18	1	Correc	
S501	Framing Details	9/4/18			
S502	Framing Details	9/4/18	1	Correc	
S503	Framing Details	9/4/18			
S601	Perspective Views	9/4/18			
S701	Simpson Strong-Wall Details	9/4/18			
S702	Simpson Strong-Wall Details (cont.)	9/4/18			
S703	Simpson Strong-Wall Details (cont.)	9/4/18			
S704	Simpson Strong-Wall Details (cont.)	9/4/18			
S705	Simpson Strong-Wall Details (cont.)	9/4/18			

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GSN - General Structural Notes

GSN - Special Inspections (Soils) 2015Image: Continuous Special InspectionPeriodic Special InspectionTypeImage: Continuous Special InspectionSpecial Inspection1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.-x2. Verify excavations are extended to proper depth and have reached proper material.-x3. Perform classification and testing of compacted fill materials.-x4. Verify use of proper materials, densities and lift thicknesses during placement and compcation of compacted fill.x-5. Prior to placement of compcted fill, inspect subgrade and verify that site has been prepared properly.-x

GSN - Inspection of Tasks Prior to Welding - 2015

Inspection Tasks Prior to weiging (AISC 360-10 Table N5.4-1)			
Inspection	Quality Control	A	
Welding procedure specifications (WPSs) available	Р		
Manufacturer certifications for welding consumables available	Р	Τ	
Material identification (type/grade)	0	Τ	
Welder identification system (The fabricator or erector, as applicable, shall maintain a system by which a welder who has welded a joint or member can be identified. Stamps, if used, shall be the low-stress type.)	0		
Fit-up of groove welds (including joint geometery			
- Joint preparation	0		
- Dimensions (alignment, root opening, root face, bevel)	0	Τ	
- Cleanliness (condition of steel surfaces)	0		
- Tacking (tack weld quality and location)	0		
- Backing type and fit (if applicable)	0		
Configuration and finish of access holes	0		
Fit-up of fillet welds			
- Dimensions (alignment, gaps at root)	0	Τ	
- Cleanliness (condition of steel surfaces)	0	Τ	
- Tacking (tack weld quality and location)	0		
Check welding equipment	0		

GSN - Inspection of Tasks During Welding - 2015

Inspection Tasks During Welding (AISC360-10 Table N5.4-2)		
Inspection of Tasks During Welding	Quality Control	A
Use of qualified welders	0	
Control and handling of welding consumables		
- Packaging	0	
- Exposure control	0	
No welding over cracked tack welds	0	
Environmental conditions		
- Wind speed within limits	0	
- Precipitation and temperature	0	
WPS followed		
- Settings on welding equipment	0	
- Travel speed	0	
- Selected welding materials	0	
- Shielding gas type/flow rate	0	
-Preheat applied	0	
- Interpass temperature maintained (min./max.)	0	
- Proper position (F, V, H, OH)	0	
Welding techniques		
- Interpass and final cleaning	0	
- Each pass within profile limitations	0	
- Each pass meets quality requirements	0	
GSN - Inspection Tasks After Welding	j - 2015	5
Inspection Tasks After Welding (Table N5.4-3)		
	Quality	

Inspection Tasks After Wolding	Quality	
inspection tasks after weiging	Control	
Velds cleaned	0	
ize, length and location of welds	Р	
Velds meet visual acceptance criteria		
Crack prohibition	Р	
Weld/base-metal fusion	Р	
Crater cross section	Р	
Weld profiles	Р	
Weld size	Р	
Undercut	Р	
Porosity	Р	
Arc strikes	Р	
-area (When welding of doubler plates, continuity plates or stiffeners has been performed in the k-area, visually nspect the web k-area for cracks within 3 in. (75 mm) of the weld.)	Р	
Backing removed and weld tabs removed (if required)	Р	
Repair activities	Р	

Р

Document acceptance or rejection of welded joint or member

GSN - Inspection Tasks Prior to Bolting - 2015 Inspection Tasks Prior to Bolting (AISC360-10 Table N5.6-1) Quality Control Assurance Inspection Tasks Prior to Bolting Manufacturer's certifications available for fastener materials Fasteners marked in accordance with ASTM requirements Proper fasteners selected for the joint detail (grade, type, bolt length 0 if threads are to be excluded from shear plane) Proper bolting procedure selected for joint detail Connecting elements, including the appropriate faying surface condition 0 and hole preparation, if specified, meet applicable requirements Pre-installation verification testing by installation personnel observed and Р documented for fastener assemblies and methods used Proper storage provided for bolts, nuts, washers and other fastener Ο components GSN - Inspection Tasks During Bolting - 2015 Inspection Tasks During Bolting (Table N5.6-2) Quality Quality Control Assurance Inspection Tasks During Bolting Fastener assemblies, of suitable condition, placed in all holes and washers (if required) are positioned as required Joint brought to the snug-tight condition prior to the pretensioning Ο operation Fastener component not turned by the wrench prevented from rotating Fasteners are pretensioned in accordance with the RCSC Specification, Ο progressing systematically from the most rigid point toward the free edges GSN - Inspection Tasks After Bolting - 2015 Inspection Tasks After Bolting (AISC360-10 Table N5.6-3) Quality Quality Control Assurance Inspection Tasks After Bolting Document acceptance or rejection of bolted connections P P GSN - Inspection Notes for Welding & Bolting - 2015 O – Observe these items on a random basis. Operations need not be delayed pending

these inspections. P – Perform these tasks for each welded joint or member.

1 Floor sheathing per plan ② Floor truss per plan (as occurs) ③ Simpson holddown strap per plan ④ Wood wall per plan Steel beam per plan Wood filler with (2) 3/4" dia thru bolts

ote	es	
g, and o	electrical	
bor bari e slab s be cut ed in sl	rier) over 4" should be #3 at control ab thickness. crete, and	
noted o	otherwise).	
ting sch	nedule	
ort or a	oplicable	
#3 ties	@ 6" O.C.	
of the p	pier (4 total).	
	ete stem wall	
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ule "		
lieu of	16d nails. Minimum Sill	
Plate loor *	Plate Thickness	
0.C.	2x	
0.C.	2x	
0.C.	2x	
0.C.	3x	
	2x	
. x 5" vs @ 6"	3x	
. x 5" vs @ 4"	3x	
. x 5" vs @ 4"	Зx	
. x 5" vs @ 4"	Зx	
S		
panel e	edges (where	
on pla ch end g walls. otherw	ns, details, or of shear wall <i>i</i> ise.	
ill plate e attac allowe	attachment hment at d by G.S.N.	
per Ge	neral Details. (staggered)	
gde na	n the sill plate	
vith a v /ided a	vidth of up to standrd cut	
unai sill conne	piate is used, ction of the	
ne sche ggered	edule. (2)2x).	
Notes		
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of wa clear f of wa	II rom soil side II	
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ng	Spacing	
su su	Equal Found	
LS LS	Equal	
LS SL	Equal	
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5	
nal Rein	forcing
ength	Spacing
1'-6"	Equal
2'-0"	Equal
2'-6"	Equal
3'-0"	Equal
3'-6"	Equal
4'-0"	Equal
4'-6"	Equal
5'-6"	Equal
6'-6"	Equal
7'-0"	Equal
<i>∕</i> ./'_∩"	Equal

S201

1/4" = 1'-0"

1 Verify all dimensions with architect prior to start of construct 2 Verify locations of all inserts in slab with the architectural, me plans prior to the placement of concrete slab. 3 Concrete slab on grade shall be 4" thick over 2" sand over 1 type II aggregate base material. Recommended reinforcer basts at 18" o.c. each way (for best crack control results, every joints). As an alternate, 6 x 6 - W2.1 x W2.1 welded wire fabri 4 Anchor bolts and holddowns shall be secured in place prior shall be installed per the manufacturers specifications and 1 5 All site walls, sidewalks, or other architectural features shall be instelled per the manufacturers specifications and 1 6 The finished surface of the slab on grade shall be noted as 'otherwise on the footing and foundation plan. 7 Size and reinforcement of isolated and continuous footings below. 8 All exterior footings shall bear below frost depth (42') accord building codes.	on. echanical, plur 0 mil. visqueer nent of the co ry other bar sh c (in sheets), c to the placer	mbing,
2 plans prior to the placement of concrete slab. 3 Concrete slab on grade shall be 4" thick over 2" sand over 7 type II aggregate base material. Recommended reinforcer bars at 18" o.c. each way (for best crack control results, ever joints). As an alternate, 6 x 6 - W2.1 x W2.1 welded wire fabri shall be installed per the manufacturers specifications and 1 4 Anchor bolts and holddowns shall be secured in place prior shall be installed per the manufacturers specifications and 1 5 All site walls, sidewalks, or other architectural features shall be noted as otherwise on the footing and foundation plan. 7 Size and reinforcement of isolated and continuous footings below. 8 All exterior footings shall bear below frost depth (42") accord building codes.	0 mil. visqueer nent of the co ry other bar sh c (in sheets), co to the placem	1 (vano
 bars at 18" o.c. each way (for best crack control results, every joints). As an alternate, 6 x 6 - W2.1 x W2.1 welded wire fabri shall be installed per the manufacturers specifications and 1 Anchor bolts and holddowns shall be secured in place prior shall be installed per the manufacturers specifications and 1 All site walls, sidewalks, or other architectural features shall be The finished surface of the slab on grade shall be noted as otherwise on the footing and foundation plan. Size and reinforcement of isolated and continuous footings below. All exterior footings shall bear below frost depth (42") accord building codes. 	ry other bar sh c (in sheets), co to the placem	ncrete
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o otherwise on the footing and foundation plan. 7 Size and reinforcement of isolated and continuous footings below. 8 All exterior footings shall bear below frost depth (42") accord building codes. Keynotes	he General No he by others (un 00'-0" (elevatio	otes. nless n on) unl
8 All exterior footings shall bear below frost depth (42") accord building codes.	shall be per the	e footi
Keynotes	ling to the soils	3 repor
Key Note Keynote Text 3109 Simpson Strong-Wall - provide Simpson SSW21x12 stem anchorage to concrete. Install per main	el Strong-Wall. nufacturer's spe	. See fo ecific <i>a</i>
3110Simpson Strong-Wall - provide Simpson SSW15x12 ster anchorage to concrete. Install per mai5401Concrete pilaster - provide (4) #4 bars vertically	el Strong-Wall. hufacturer's spe full wall height	See fo ecific <i>a</i> with #
5402Concrete pier - provide 12"x12" with (1) #4 vertical B Provide #3 ties @ 4" o.c. full6101Provide 11"x11"x3/4" base plate with (4) 3/4" dia	bar at each co height of pier. Imeter anchor	orner o bolts i
6102 Provide 5½"x11"x3/4" base plate with (2) 3/4" diamet	er anchor bolt	s into c
Plywood Shear Wall	Sche	du
* Where 1-1/8" plywood is used for floor sheathing, use	1/4"x4" screv	vs in li
Mark Sheathing Thickness Edge Nailing A.B. Spa	acing @ Bot Bot Bation Nail	itom Pl Is @ Flc
SW1 3/8 plywood, blocked, one side of wall 8d @ 6" o.c. 1/2 dia bolts @ SW2 3/8" plywood, blocked, one side of wall 8d @ 4" o.c. 1/2" dia bolts @	32" o.c. 16c anchor 16c 22" o.c. 16c	1@6"
SW3 SW3 <td>anchor 10" o.c.</td> <td>1@3"</td>	anchor 10" o.c.	1@3"
SW3 (alt)3/8" plywood, blocked, one side of wall, 3" nominal framing @ panel edges8d @ 3" o.c.1/2" dia bolts @	anchor 20" o.c. 16c	1@3"
SW43/8" plywood, blocked, one side of wall, 3" nominal framing @ panel edges8d @ 2" o.c. bolts @1/2" dia bolts @	anchor 8" o.c.	
SW4 (alt)3/8" plywood, blocked, one side of wall, 3" nominal framing @ panel edges8d @ 2" o.c.	1/4 Iong	l" dia. screw: o.c.
SW5	anchor 16" o.c. long	l" dia. screws o.c.
3/8" plywood, blocked, both 8d @ 3" o.c. 5/8" dia SW6 side of wall, 3" nominal framing (staggered) bolts @	anchor 16" o.c. long	l" dia. screw: o.c.
SW73/8" plywood, blocked, both side of wall, 3" nominal framing @ panel edges8d @ 2" o.c. (staggered)5/8" dia bolts @	anchor 16" o.c. long	l" dia. screw: o.c.
Plywood Shear Wa	all No	tes
A. Brovide (2) full height studs (min.) at ends of shear walls, unle	ess noted other	ig at p iwise. irwise c
(min.) Trimmer stud may be counted as an end C. Plywood may be installed either horizontally or ver Use A.B. (Anchor Bolts) noted per G.S.N. details, and this so	stud at non-b ically, unless n	
D. at foundation. Use staggered nails as noted in this sched elevated shear walls. See G.S.N. for optional shot-pins and E. Where sheathing is interrupted by intersecting wall, provide	epoxy bolts w) plate /here a
F. Multiple 2x studs at holddowns shall be stitch-nailed togethe G. All field nailing shall be at 12" o.c. with the same size	r with 16d sink nail specified	ers at of for eg
Anchor bolts for shear walls shall include steel plate washers, and the nut. The hole in the plate washer is permitted to be 3/16" larger than the bolt diameter and a slot length not to washer is placed between the plate washer and the put. W	.229"x3"x3", in diagonally slot exceed 1-3/4"	size be tted w ', provi " pomi
26 103 (2)20d box nails shall be substituted for (2)16d commend r stud to the sole plate (2)2x framing may be used in lieu of the 2x period frame.	ails for the end $\frac{1}{2}$	
J. (2)2X training may be used in field of the sX hominan training framing shall be stitch-nailed together with 16d s	inkers @ 6" o.c	. (stag
	hedu	le
Mark Wall Ventical Holizontal Top & Bo Mark Thickness Reinforcing Reinforcing Bar CW1 8" #4@12" o.c. #4@12" o.c. (2)#4	s pars	Cente
CW2 8" #5@12" o.c. #5@12" o.c. (2)#5 B CW4 8" #5@8" o.c. #5@12" o.c. (2)#5 B	pars Provi	de 6" (ide 6" (
	Scho	d
Mark Width Thickness Lc		nforcin
FC10 1' - 0" 2 ## S301 FC16 1' - 0" 3 ##	Cont Cont	tinuous
FC26 2' - 6" 1' - 0" 4 #4 FC30 3' - 0" 1' - 0" 5 #4 FC36 3' - 6" 1' - 0" 6 #4	Cont Cont	tinuou: tinuou: tinuou:
Isolated Footing Se	-hedi	ule
Mark Width Length Thick- ness Crosswise Reinforcing No. Size Length Spacing	Long ng No. Siz	gitudin ze Le
FS20 2' - 0" 12" 3 #4 1'-6" Equal FS26 2' - 6" 12" 4 #4 2'-0" Equal FS30 3' - 0" 3' - 0" 12" 5 #4 2'-6" Equal	al 3 #4 al 4 #4	4 4 4
FS36 3' - 6" 12" 6 #4 3'-0" Equal FS40 4' - 0" 12" 6 #4 3'-6" Equal FS40 4' - 0" 12" 6 #4 3'-6" Equal	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\frac{4}{4}$
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	al7# al 8# al 9#	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$
FS70 7' - 0" 7' - 0" 12" 11 #4 6'-6" Equal FS76 7' - 6" 7' - 6" 12" 12 #4 7'-0" Equal FS4686 4' - 6" 8' - 6" 12" 7 #4 8'-0" Equal	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4 0 4 4
		<u>.</u>
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834 West 75 North Kaysville, UT 84037 (phone) 801.915.4525 www.5iveengineering.com These construction documents are provided as an instrument of service, and are the propery of 5ive Inc. Use of these construction documents and all eproductions shall be restricted to the original site for which they were prepared, and shall not be used unless properly approved and sealed. Publication of these construction documents by any method, in whole or ir part, is prohibited without written consen by 5ive Inc. Where a contractors name appears on these construction documents, work shall be performed by said contractor, except as agreed upon Description 00 51 REVIEWED FOR CODE COMPLIANCE OCOMPLIANCE DO COMPLIANCE WITH THE APPLICAL INSTRUCTION CODES IDENTIFIED BEI BUILDING STRUCTUR IMECHANICAL STRUCTUR IMECHANICAL ENERGY ACCESSIBILITY FIRE PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS. 8/27/2019 10:12:32 AM Main Level Footing & Foundation Plan 9/4/18 BPT BPT S202 1/4" = 1'-0"

Scale

Wood wall per plan
Concrete slab on grade (see General Structural Notes for thickness & reinforcement)
Concrete footing (see plan for
size & reinforcing)
Wood Wall @ Concrete Footing 1.01 1" = 1'-0"

H	Flo	or & Low Ro	oof Fra	imina F	lan No	tes
	1	Roof sheathing shall be 3/4" A	A.P.A. rated shea attachn /8" A P A rated	athing. See Gener nent.	al Details for diaph	iaphragm
	2 FI	Floor framing members shown or m	on plan indicate	e manufactured fla "o.c. typical".	Sor trusses spaced	per the
	Typ 16 4 pla	pical joint splice of the perimeter of each side of the joint. Nails sha ate splice shall be located away	(2) 2x top plate all be staggered from marked sh	e shall have a mini I and evenly spac nearwalls. Where i	mum of 4'-0" overla ed (see General D t is not feasible to c	ap with (20) etails). Top overlap the
	5 of	l exterior walls, and interior beari	ng walls, shall be	e framed with 2x6	studs at 16" o.c. ur	e, and fully nless noted
	6 "I	D=xxx#" indicates drag load to b	wall wall we included in th manufac	he appropriate loa	an openings in woo	y the truss
				tes		
	Key Note	e Simpson Strong-Wall - provid	Keyr de Simpson SSW	note Text 21x12 steel Strong	-Wall. See foundati	ion plan for
	3107	anchorage to Simpson Strong-Wall - provio anchorage to	concrete. Instal de Simpson SSW concrete. Instal	l per manufacture 15x12 steel Strong I per manufacture	r's specifications. -Wall. See foundati er's specifications.	ion plan for
	3113 3114	Simpson Strong-Wall - provid anchorage to Simpson Strong-Wall - provid	de Simpson SSW concrete. Instal de Simpson SSW	21x10 steel Strong per manufacture 24x10 steel Strong	-Wall. See foundati r's specifications. -Wall. See foundati	ion plan for ion plan for
	4107	Wood ledger - provide	1-3/4"x11-7/8" LV	Liedger with (8)#	10 wood screws @	16" o.c.
	* Whei	re 1-1/8" plywood is used for	floor sheathin	Vall SCI ng, use 1/4"x4" s	screws in lieu of	16d nails.
	Mark	Sheathing Thickness	Edge Nailing	Bottom Plate A.B. Spacing @ Foundation	Attachment Bottom Plate Nails @ Floor *	Minimum Sill Plate Thickness
	SW1	3/8" plywood, blocked, one side of wall 3/8" plywood, blocked, one	8d @ 6" o.c.	1/2" dia. anchor bolts @ 32" o.c. 1/2" dia. anchor	16d @ 6" o.c.	2x
	SW2 SW3 s	side of wall 3/8" plywood, blocked, one side of wall, 3" nominal framing	8d @ 3" о.с.	bolts @ 32" o.c. 1/2" dia. anchor	16d @ 6" o.c.	2x 2x
	SW3 s	@ panel edges 3/8" plywood, blocked, one side of wall, 3" nominal framing	8d @ 3" o.c.	1/2" dia. anchor	16d @ 3" o.c.	3x
$\searrow_{i=1}^{S}$	(ait) SW4 s	@ panel edges 3/8" plywood, blocked, one side of wall, 3" nominal framing	8d @ 2" o.c.	1/2" dia. anchor		2x
	SW4 (alt) s	@ panel edges 3/8" plywood, blocked, one side of wall, 3" nominal framing	8d @ 2" o.c.		1/4" dia. x 5" long screws @ 6"	3x
-(2)CS16	SW5 s	@ panel edges 3/8" plywood, blocked, both side of wall, 3" nominal framing	8d @ 4" o.c. (staggered)	5/8" dia. anchor bolts @ 16" o.c.	0.C. 1/4" dia. x 5" long screws @ 4"	3x
	SW6 s	@ panel edges 3/8" plywood, blocked, both side of wall, 3" nominal framing @ panel edges	8d @ 3" o.c. (staggered)	5/8" dia. anchor bolts @ 16" o.c.	0.c. 1/4" dia. x 5" long screws @ 4"	3x
	SW7 s	3/8" plywood, blocked, both side of wall, 3" nominal framing @ panel edges	8d @ 2" o.c. (staggered)	5/8" dia. anchor bolts @ 16" o.c.	1/4" dia. x 5" long screws @ 4" o.c.	Зx
		Plywood	Shear	WallN	lotes	
(2)CS16	A. Shea	ar wall studs shall be placed at noted above) s vide (2) full height studs (min.) at d down requirements. Shear wal	6" o.c. maximul hall be 2x minim ends of shear v	m. Framing and bl num, unless noted valls, unless noted	ocking at panel ec otherwise. otherwise on plan:	s, details, or
$\left(\begin{array}{c} 232\\ 5503\end{array}\right)$	C.	(min.) Trimmer stud ma Plywood may be installed	y be counted a either horizonta	s an end stud at n lly or vertically, un	ion-bearing walls. less noted otherwis	se.
	D. E. Wh	at foundation. Use staggered na levated shear walls. See G.S.N. for ere sheathing is interrupted by in	ails as noted in the properties of the propertie	his schedule for bo pins and epoxy b provide continuity	ottom plate attach otts where allowed v channel per Gen	ment at by G.S.N. eral Details.
	F. Mul G.	Itiple 2x studs at holddowns shall All field nailing shall be at 1: thor bolts for shear walls shall incl	be stitch-nailed 2" o.c. with the s	I together with 160 same size nail spec	d sinkers at 6" o.c. (s cified for egde naili 3", in size between	staggered). ing. the sill plate
<u>1</u>	H. was	d the nut. The hole in the plate w 16" larger than the bolt diameter sher is placed between the plate	and a slot leng washer and th	ed to be diagonal th not to exceed ² e nut. Where a sin	ly slotted with a wid 1-3/4", provided a s gle 3" nominal sill p	dth of up to standrd cut late is used,
	(2	2)20d box nails shall be substitute 2)2x framing may be used in lieu	d for (2)16d con stud to the so of the 3x nomin	nmend nails for th ole plate. nal framing is calle	e end nail connect	tion of the Jule. (2)2x
		traming shall be stitch-na	alled together v	vith 16d sinkers @ 6	o o.c. (staggered).]
			TION			
		IKE PROTEC				
	- -		UIKEL	ΎΙ		
		NFPA 13 NFPA 13D				
		NFPA 13R				
		Other				
(H)						

BPT

BPT

1/4" = 1'-0"

Drawn By

Scale

Checked By

S401

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al deta st open	ils. ing over the
Strong n SDS2!	-walls. 5600 wood
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lle	
lieu of	f 16d nails.
ent Plate	Minimum Sill Plate
oor *	Thickness
0.C.	2x
0.C.	2x
0.C.	2x
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	Ç.
<u>у</u> Б"	Ζλ
vs @ 6"	Зх
x 5" vs @ 4"	Зx
x 5"	
vs @ 4"	3х
x 5" vs @ 4"	3x
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) panel @	edges (where
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per Ge 6" o.c.	neral Details. (staggered).
gde na etwee	illing. n the sill plate
vith a v vided a	vidth of up to standrd cut
iinal sill conne	plate is used, ction of the
ne sche	edule. (2)2x
ggered).

FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW. BUILDING STRUCTURAL MECHANICAL PLUMBING ELECTRICAL ENERGY ACCESSIBILITY FIRE PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OB LOCAL REGULATIONS. MEST COAST CODE CONSULTANTS, INC.

Date 9/4/18 Drawn By BPT Checked By BPT **S402**

1/4" = 1'-0"

231) Floor Truss @ Wood Beam 1.01 1" = 1'-0"

232 Floor Truss @ Concrete Wall 2.01 1" = 1'-0"

233 Steel Beam @ Slanted Column 1" = 1'-0"

2 Front Right

4 Back Right

and the second se	CONCRETE	ANCHOR	ALL ON MADIE	W	de	ASD	W	de		
OKITERIA	CONDITION	STRENGTH	UPLIFT (lbs) 8,700	(in) 18	(in)	UPLIFT (lbs) 16.000	(in) 27	(in) 9	-	
	CRACKED	STANDARD	9,600	20 29	7	17,100 32,100	29 42	10 14	-	
SEISMIC			19,900 9,100	32 16	11 6	35,300 15,700	45 23	15 8	-	
	UNCRACKED		9,600 17,800	17 25	6 9	17,100 32,500	25 37	9 13		
		HIGH STRENG	19,900 5,400	27 12	9 6	35,300 6,800	39 14	13 6		
	in the second	STANDARD	8,300 9,600	16 18	6 6	11,600 17,100	20 26	7 9	-	
	CRACKED	HIGH STRENG	11,600 13,400	20 22	7 8	21,400 25,800	30 34	10 12	-	
WND	-		17,300	26 29	9	31,000 35,300	38 42	13	-	
		STANDARD	8,500 9,600	12	6	12,400	12 18 23	6	-	
	UNCRACKED		12,400	18	6 7	21,600 26,700	26 30	9	-	
		HIGH STRENG	ГН <u>16,800</u> 19,900	22 25	8 9	32,200 35,300	34 36	12 12]	
										3
1	0510111-3	S	TEEL STRONG-WAL	L SHEAR ANCH	HORAGE	//NID4				
	SEISMIC				N	ASD ALLOWAE	3LE SHE/	AR LOAD V (Ib	s .) ⁶	
L _t OR L _n (in.)	SHEAR REINFORCEMENT	MIN. CURB/ STEMWALL	SHEAR REINFORCEMENT	MIN. CURB / STEMWALL	6" MIN CUR	B/STEMWALL		8" MIN CURB	/STEMWALL	
					UNCRACK	ED CRACK	ED U		CRACKED	
9 12	(1) #3 TIE (2) #3 TIES	ю 6	NONE REQUIRED	-	1230	1135		1440	1295	
14	(1) #3 HAIRPIN	85	(1) #3 HAIRPIN	6						
				in the second se	HAIRPIN R	FINFORCEMEN	I ACHIE	VES MAXIMUN	VI ALLOWABLE	20
15 17 NOTE 1. S 2. S 2. S 4. V 5. M 6. U 7. C	(2) #3 HAIRPIN (2) #3 HAIRPIN (2) #3 HAIRPIN S: HEAR ANCHORAG EE DETAILS 1/SSV HEAR REINFORCE PPLICATIONS (PA EISMIC INDICATES SE WIND ANCHOF VIND INCLUDES SE UNIMUM CURB/ST SE (1) #3 TIE FOR ABULATED ANCHO ONCRETE EDGE I	8 ⁵ BE DESIGNS CO W1 TO 3/SSW1 F EMENT IS NOT F NEL INSTALLED S SEISMIC DESI RAGE SOLUTION EISMIC DESIGN EMWALL WIDTH SSW12 AND SS ORAGE ALLOW/ DISTANCE FOR	(1) #3 HAIRPIN (1) #3 HAIRPIN (1) #3 HAIRPIN NFORM TO ACI 31 FOR TENSION AND REQUIRED FOR PA O AWAY FROM EDO GN CATEGORY C NS. CATEGORY A AND IS 6" WHEN STAN W15 WHEN THE S ABLE SHEAR LOAD ANCHORS MUST (6 8-14 AND ACI CHORAGE. ANELS INSTAL GE OF CONCR THROUGH F. D B. IDARD STREN IDARD STREN COMPLY WITH	SHEA 318-11 AND LED ON A W RETE), OR BF DETACHED NGTH SSWAE G-WALL PAN H ACI 318-14	R LOAD OF THI ASSUME MIN /OOD FLOOR, RACED WALL 1 AND 2 FAM B IS USED. IEL DESIGN S SECTION 17.7	E STEEL	STRONG-WAL C=2,500 PSI C OR FOUNDA APPLICATION ELLINGS IN S ORCE EXCEN ACI 318-11 E	L PANEL CONCRETE. TION NS. SDC C MAY EDS THE 0.8.2.	
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	ELECTRICAL LEGEND
SYMBOL S	ТҮРЕ
	SINGLE POLE TOGGLE SWITCH
	THREE WAY TOGGLE SWITCH
	FOUR WAY TOGGLE SWITCH
	GARAGE DOOR OPENER
	110 V DUPLEX OUTLET ON AN (AFP) ARC FAULT PROTECTED CIRCUIT
	110 V GROUND FAULT INTERRUPTER
φ	110 V WATERPROOF GFI OUTLET
220	110 V FLOOR DUPLEX OUTLET
	220 V OUTLET
S	110 V SMOKE DETECTOR W/BATT BACK-UP
©	CARBON MONOXIDE DETECTOR
R	4" LED RECESSED CAN (FIXTURE & TRIM PER SCHEDULE)
C	4" LED RECESSED CAN (CLOSET-FIXTURE & TRIM PER SCHEDULE)
\mathbb{W}	RECESSED CAN (WET LOCATION-FIXTURE & TRIM PER SCHEDULE)
	CEILING MOUNT FIXTURE
T	
	TRACK LIGHTING
S	EXHAUST FAN
\square	EXHAUST FAN WITH LIGHT FIXTURE
	2X2 OR 2X4 FLUORESCENT CEILING FIXTURE
	FLUORESCENT STRIP LIGHT
	LED UNDERCOUNTER LIGHTING
G	GARAGE DOOR OPENER
К	KEYLESS GARAGE DOOR OPENER
В	DOORBELL
	TELEPHONE (CAT 5E WIRING) SINGLE LINE UNLESS NOTED (NUMBER) DESIGNATES PORT OUTLETS REQUIRED
	MULTI-MEDIA NETWORK OUTLET (CAT 5E WIRE) W/(4) PORT OUTLET
	STRUCTURED WIRING (FUTURE SMART WIRING) IE (2) RG6 QUAD SHIELD, (2) CAT 5E WIR FOR CABLE TV, VIDEO, SATELITTE, ETC. (6) PORT OUTLET
\oplus	GARBAGE DISPOSAL
-ф-	LED PUCK LIGHT
ELECTR	
	PROJECT KEYNOTES
01-04	ALL CONSTRUCTION SHALL CONFORM TO ALL 2017 NATIONAL ELECTR CODE (N.E.C.). UTAH AMMENDMENTS, LOCAL, AND RELATED BUILDING
0/ 00/	AND STD. CONST. PRACTICES IN EFFECT.
26-00A	THE ELECT. STREW SHALL COMPLET WITH 2013 I.K.C. AND 2017 N.E.C. A
26-00A 26-00B	INSTALLED IN STRICT ACCORDANCE WITH ALL CODES. THE ELECT. CONTRACTOR SHALL BE RESPONSIBLE FOR THE COMPLETE
26-00A 26-00B 26-00C	INE ELECT. STSTEM SHALL COMPLY WITH 2013 LK.C. AND 2017 N.E.C. A INSTALLED IN STRICT ACCORDANCE WITH ALL CODES. THE ELECT. CONTRACTOR SHALL BE RESPONSIBLE FOR THE COMPLETE ELECTRICAL SYSTEM. ALL ELECT. DWGS. ARE DIAGRAMMATIC. LOCATIONS SHALL BE PER CO
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26-00A 26-00B 26-00C 26-01A 26-02A 26-02B	INE ELECT. STSTEM SHALL COMPLET WITH 2013 LK.C. AND 2017 N.E.C. A INSTALLED IN STRICT ACCORDANCE WITH ALL CODES. THE ELECT. CONTRACTOR SHALL BE RESPONSIBLE FOR THE COMPLETE ELECTRICAL SYSTEM. ALL ELECT. DWGS. ARE DIAGRAMMATIC. LOCATIONS SHALL BE PER CO OWNER. ELECT. U-FER GROUND AS REQUIRED. ALL FIXTURES SHALL HAVE A U.L. LABEL LISTING. ALL LAMPS PERMANENTLY INSTALLED SHALL BE LED LAMPS AT K TEMP. /
26-00A 26-00B 26-00C 26-01A 26-02A 26-02B 26-03A	 THE ELECT. STRIKT SHALL COMPLET WITH 2013 LK.C. AND 2017 N.E.C. AND 2
26-00A 26-00B 26-00C 26-01A 26-02A 26-02B 26-03A 26-03B 26-03D	 IHE ELECT. STSTEM SHALL COMPLET WITH 2013 LK.C. AND 2017 N.E.C. A INSTALLED IN STRICT ACCORDANCE WITH ALL CODES. THE ELECT. CONTRACTOR SHALL BE RESPONSIBLE FOR THE COMPLETE ELECTRICAL SYSTEM. ALL ELECT. DWGS. ARE DIAGRAMMATIC. LOCATIONS SHALL BE PER COOWNER. ELECT. U-FER GROUND AS REQUIRED. ALL FIXTURES SHALL HAVE A U.L. LABEL LISTING. ALL LAMPS PERMANENTLY INSTALLED SHALL BE LED LAMPS AT K TEMP. / OWNER. GFI & GFCI RECEPTACLES, SEE KEYNOTE E6/G002. SWITCHES, RECEPTACLES, TELEPHONE JACKS & CATV JACKS. HARD WIRED SMOKE DETECTORS TO BUILDING CIRCUIT AND INTERCOMMERCING.
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9. ALL BRANCH CIRCUITS BE PROTECTED BY AN ARCH-FAULT CIRCUIT INTERRUPTER LISTED TO PROVIDE PROTECTION OF THE ENTIRE BRANCH CIRCUIT.
10. PROVIDE A U-FER GROUND. AN ELECTRODE ENCASED BY A LEAST 2" OF CONCRETE SHALL BE LOCATED NEAR THE BOTTOM OF THE CONCRETE FOUNDATION SYSTEM AND SHALL BE IN DIRECT CONTACT WITH THE EARTH, CONSISTING OF AT LEAST 20 FEET OF BARE ELECTRICALLY CONDUCTIVE ROD AT LEAST 1/2 INCH IN DIAMETER OR BARE COPPER CONDUCTOR NOT SMALLER THAN 4 AWG. (I.R.C. E3508.1.2 AND N.E.C.

250.50) 11. THE CONTRACTOR SHALL VERIFY OUTLET LOCATIONS AND VOLTAGE REQUIREMENTS AS PER APPLIANCE SPECIFICATIONS.

12. STRUCTURED WIRE MEDIA PANEL TO BE "LEVITON" (O.A.E.) AND INCLUDE: A/C POWER MODULE, CAT 5 VOICE AND DATA MODULES, 10/100 MPS SATA HUB, CATV BOOSTER AND AUDIO / VIDEO CONTROL MODULES.

13. SMOKE AND/OR CARBON MONOXIDE DETECTORS ARE TO BE HARD WIRED TOGETHER IN SERIES WITH BATTERY BACKUP AS PER CODE REQUIRMENTS. COMBINATION UNITS ARE PERMITTED AS APPROVED.

14. ALL EXTERIOR ELECTRICAL OUTLETS TO HAVE WEATHERPROOF COVERS.

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15. ALL 125V 15 AND 20 AMP RECEPTACLES WITHIN DWELLING UNITS MUST BE TAMPER PROOF.

